TOWNSHIP OF MUSKOKA LAKES

LITTLE BURGESS GENERATING STATION

1 BAILEY STREET
PORT CARLING, ONTARIO

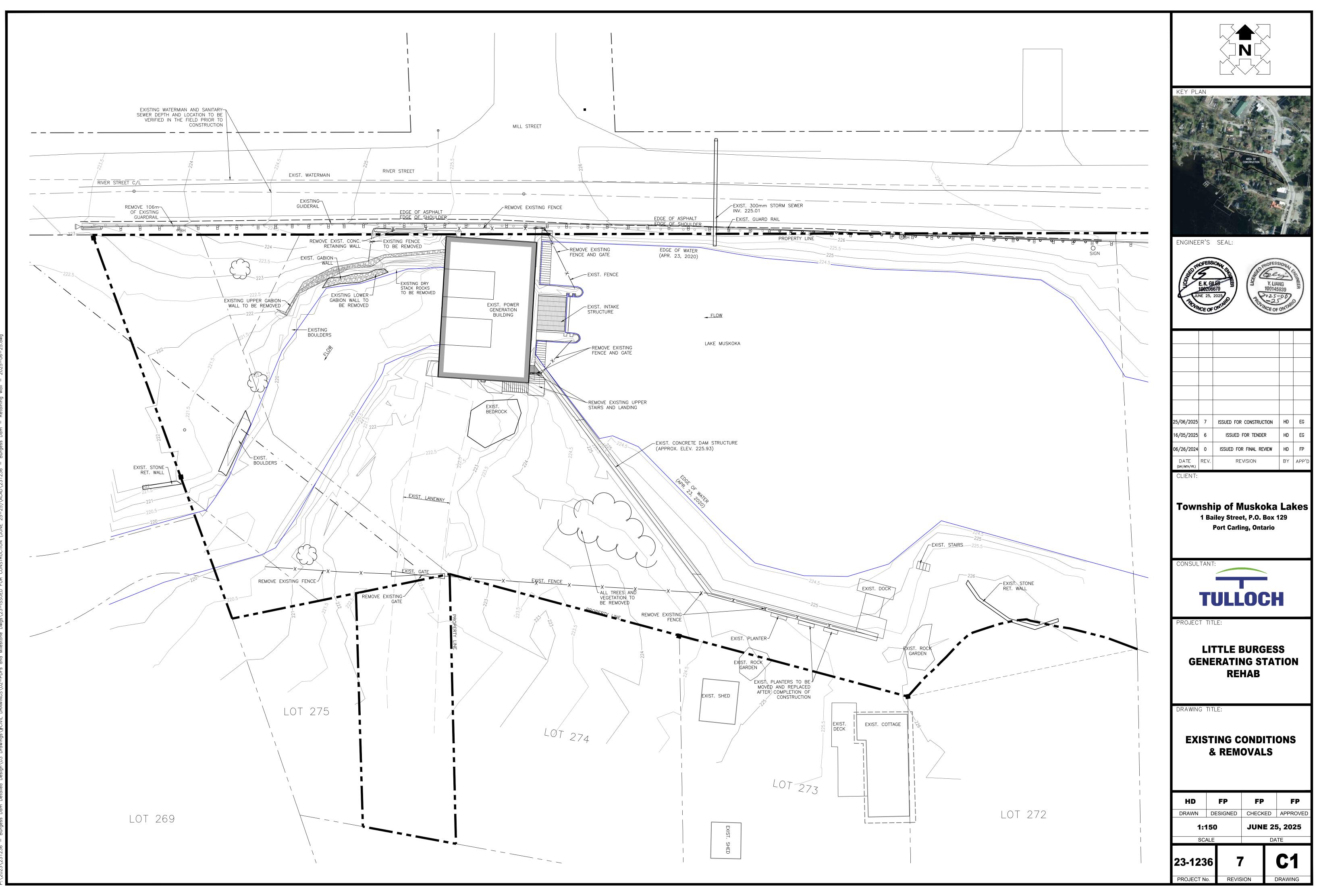


KEY PLAN SCALE=1:1000

LIST	OF DR	AWINGS
No.	Rev.	DRAWING DESCRIPTION
C1	7	Existing Conditions & Removals
C2	7	Spillway Plan
С3	7	Spillway Sections & Details
C4	7	Spillway, Dam Raise & Dam Extension Plan
C5	7	Spillway, Dam Raise & Dam Extension Section & Details
C6	7	Concrete Embankment Wall
C7	7	Laneway Grading & Details
C8	7	River Street Road Reconstruction Plan & Profile Sta. 0+940 to 1+091
E1	7	Sediment & Erosion Control Plan
S1	3	General Arrangement, Elevations & Retaining Wall Detail
S2	3	Retaining Wall Sections
S3	3	Foundation Upgrades & Roof Framing Plans
S4	3	Foundation Upgrades & Roof Framing Details
S5	3	Removal Photos
G1	7	General Notes



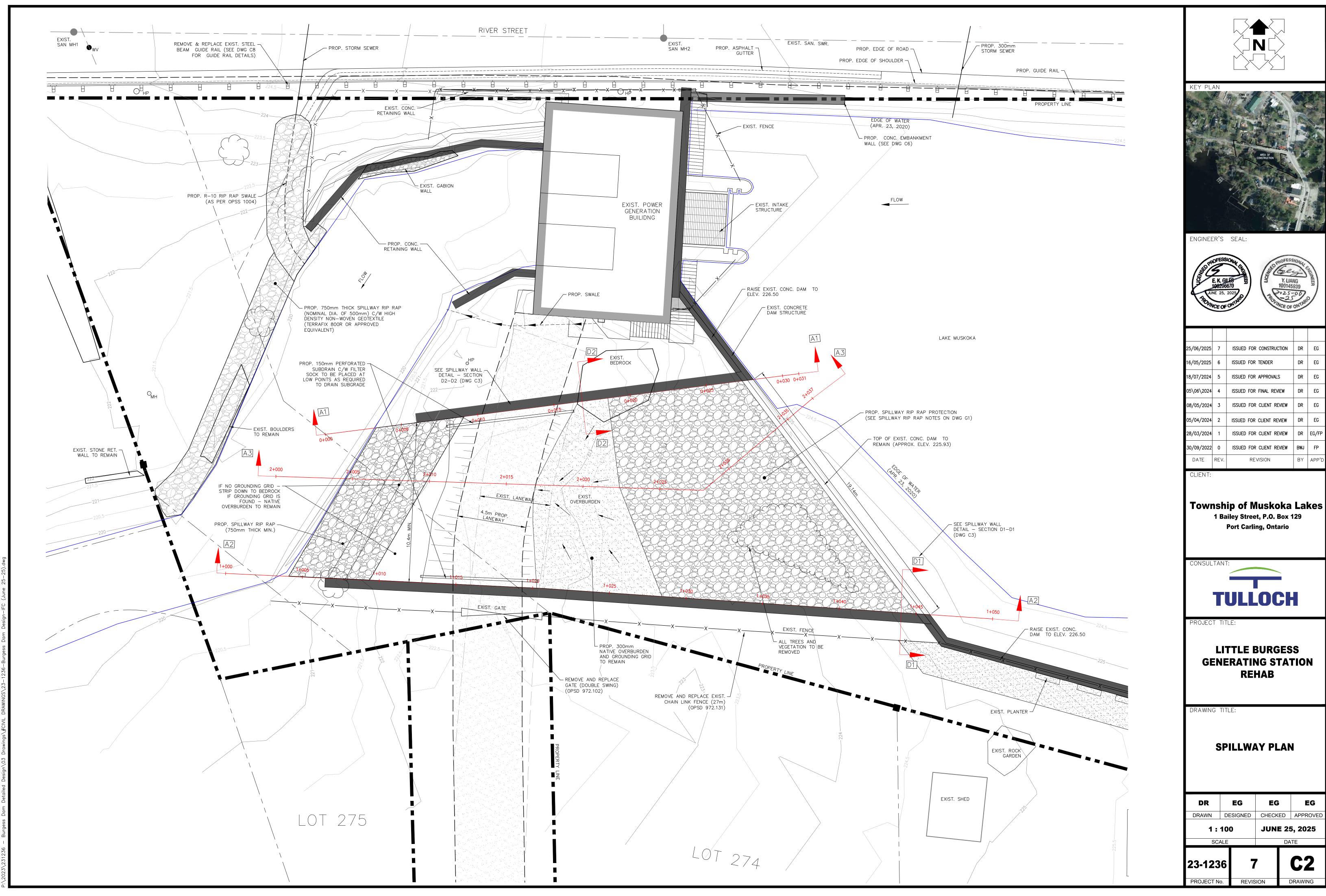






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16/05/2025	6	ISSUED FOR TENDER	HD	EG
06/26/2024	0	ISSUED FOR FINAL REVIEW	HD	FP
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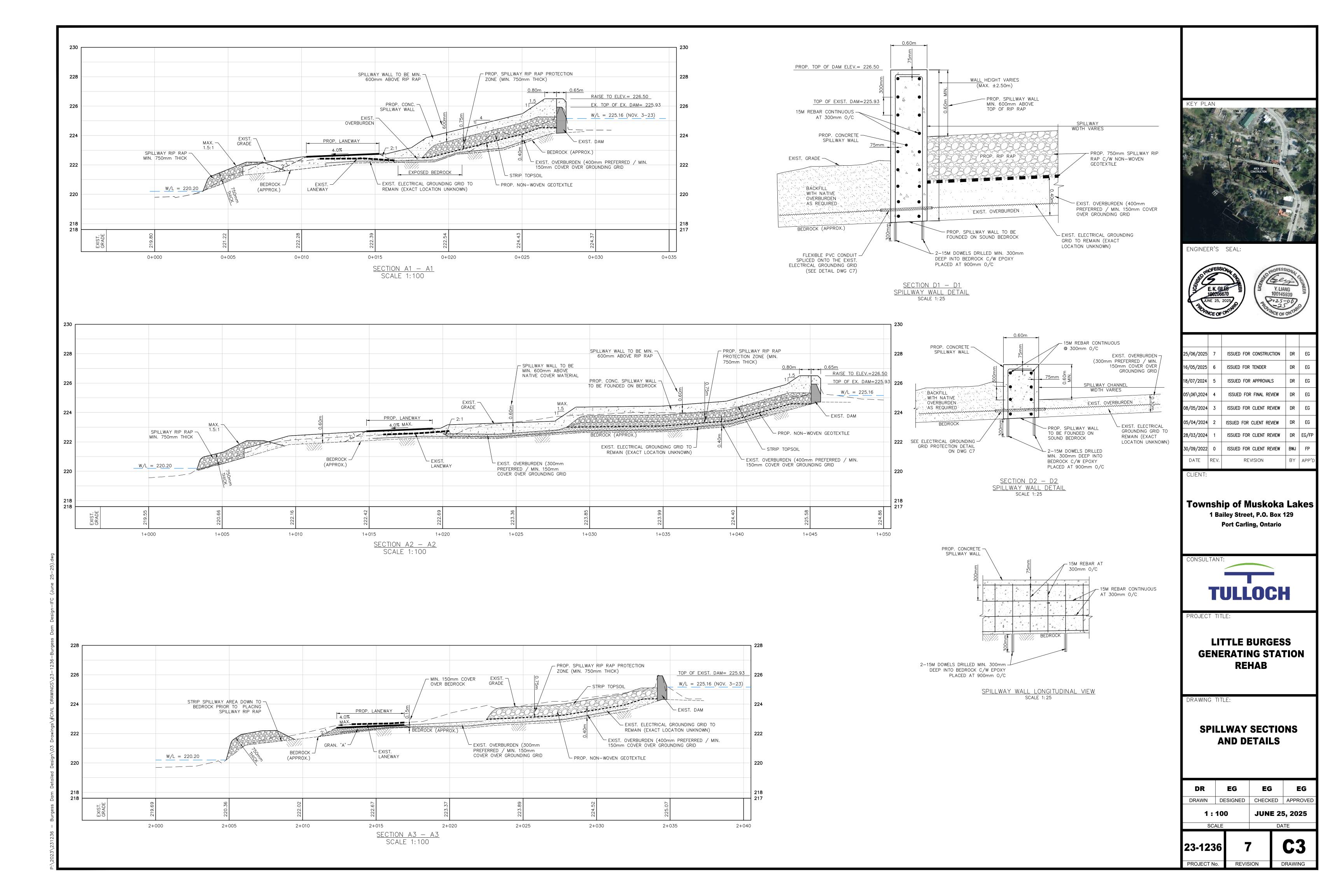
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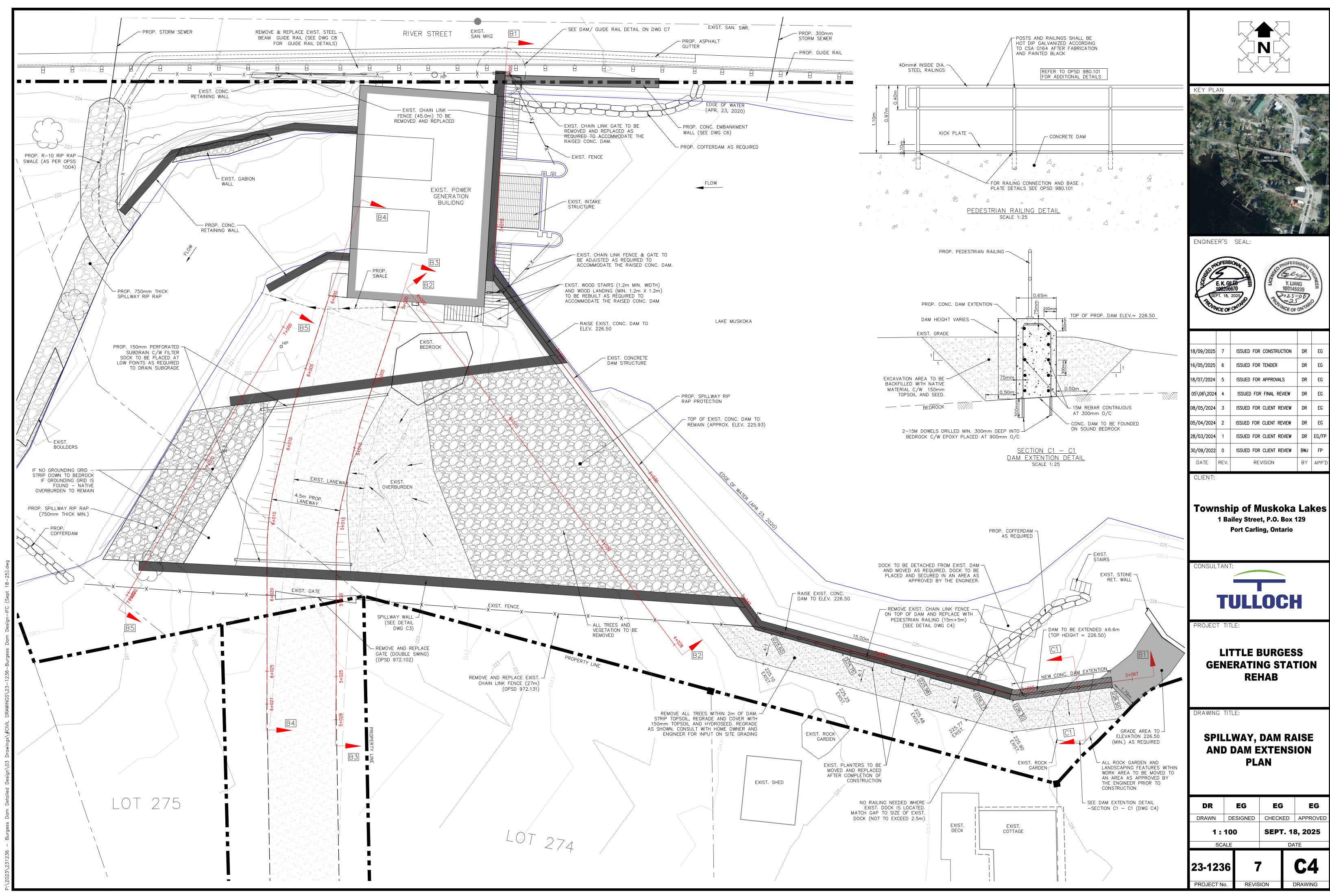




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16/05/2025	6	ISSUED FOR TENDER	DR	EG
18/07/2024	5	ISSUED FOR APPROVALS	DR	EG
05\06\2024	4	ISSUED FOR FINAL REVIEW	DR	EG
08/05/2024	3	ISSUED FOR CLIENT REVIEW	DR	EG
05/04/2024	2	ISSUED FOR CLIENT REVIEW	DR	EG
28/03/2024	1	ISSUED FOR CLIENT REVIEW	DR	EG/FP
30/09/2022	0	ISSUED FOR CLIENT REVIEW	BWJ	FP
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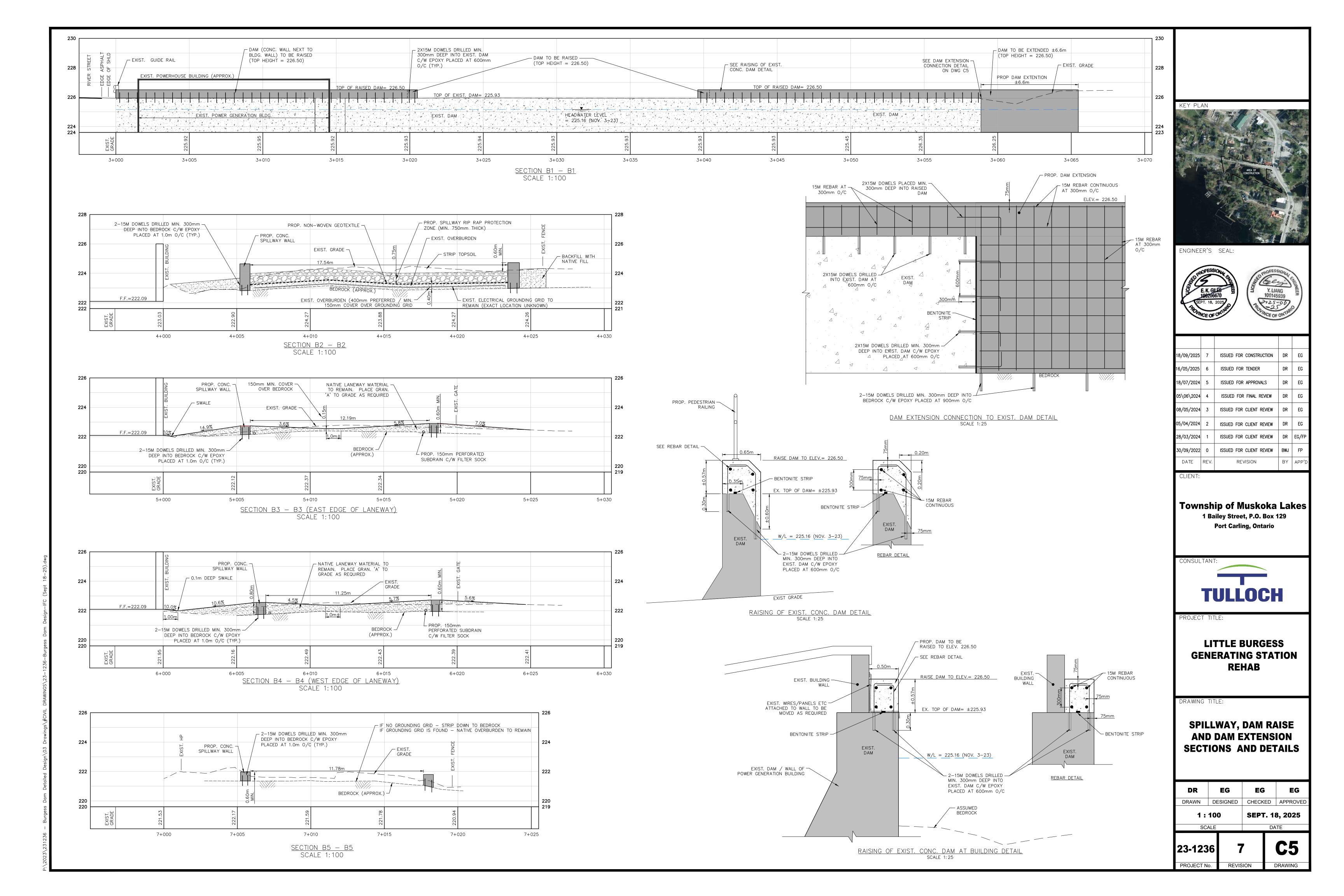
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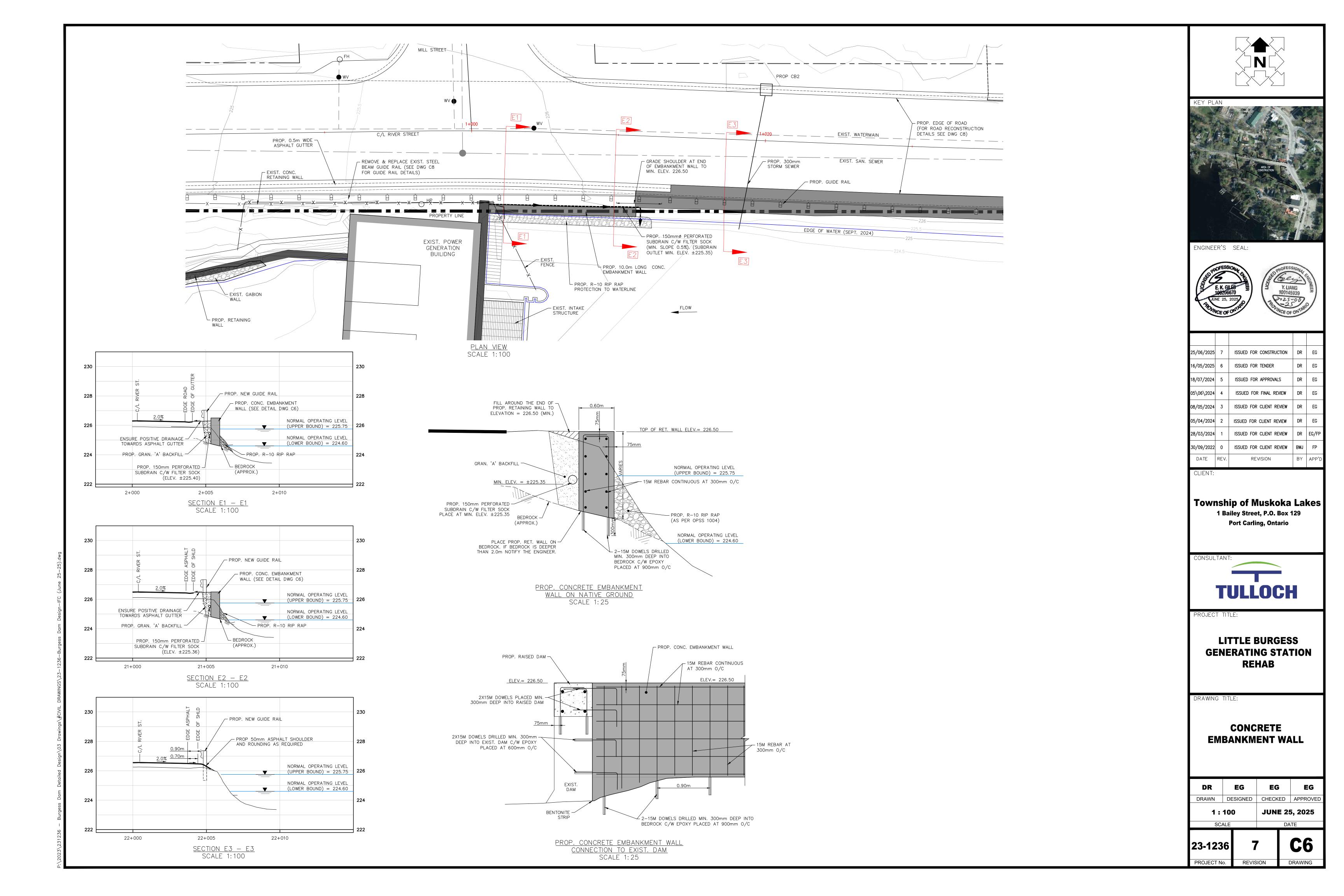


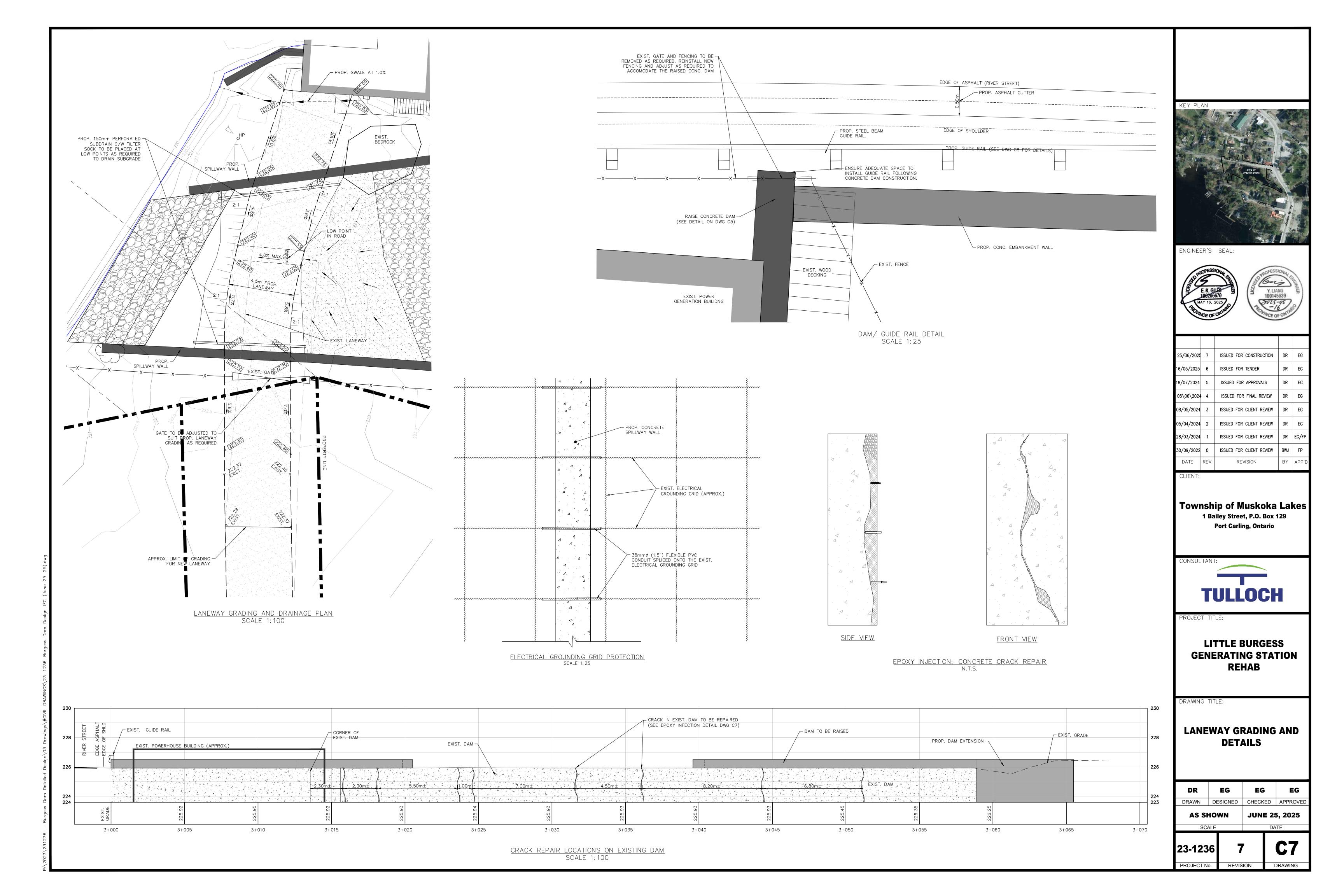


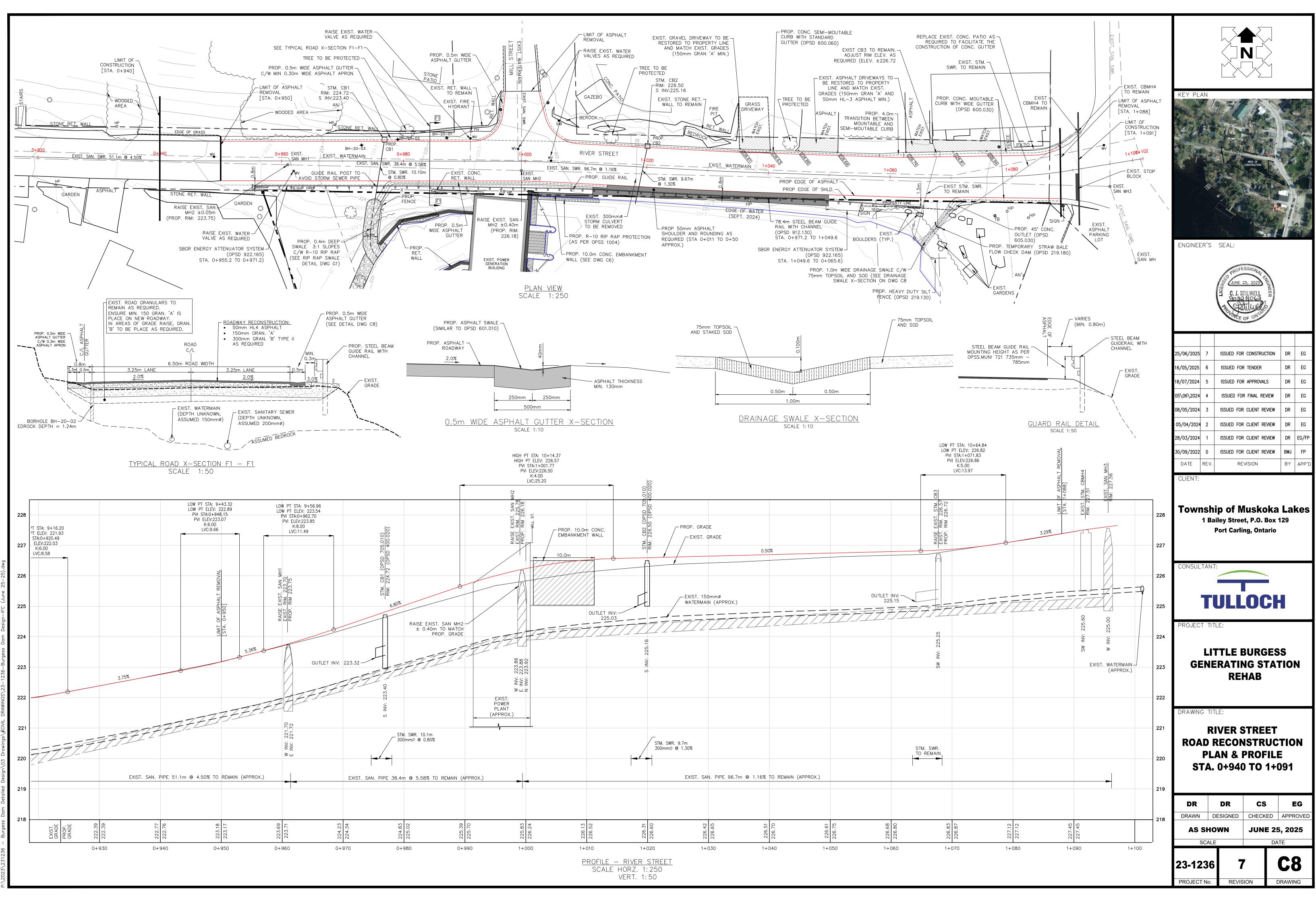


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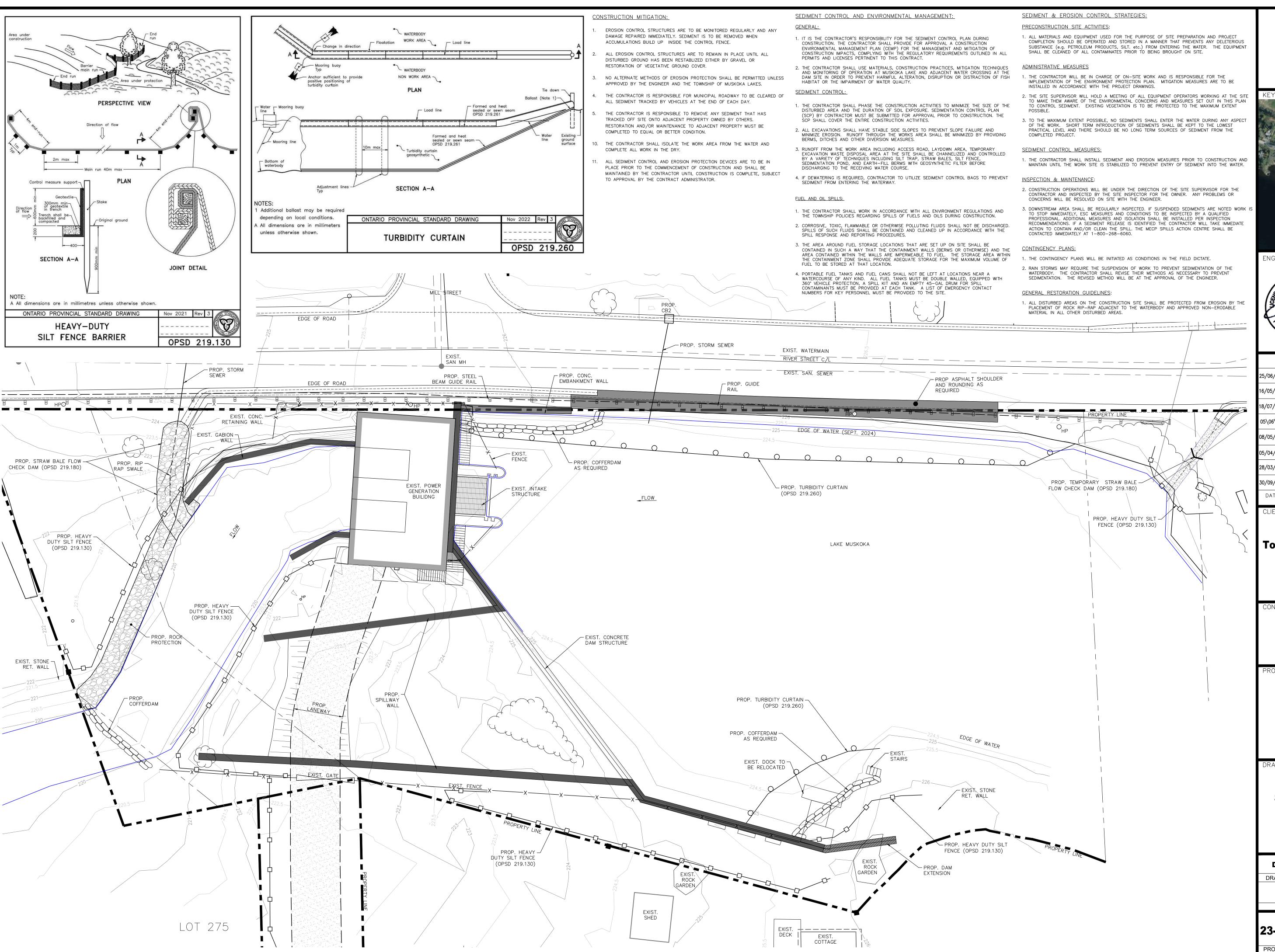








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ENGINEER'S SEAL:





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18/07/2024	5	ISSUED FOR APPROVALS	DR	EG
05\06\2024	4	ISSUED FOR FINAL REVIEW	DR	EG
08/05/2024	3	ISSUED FOR CLIENT REVIEW	DR	EG
05/04/2024	2	ISSUED FOR CLIENT REVIEW	DR	EG
28/03/2024	1	ISSUED FOR CLIENT REVIEW	DR	EG/FP
30/09/2022	0	ISSUED FOR CLIENT REVIEW	BWJ	FP
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Township of Muskoka Lakes 1 Bailey Street, P.O. Box 129 **Port Carling, Ontario**



PROJECT TITLE:

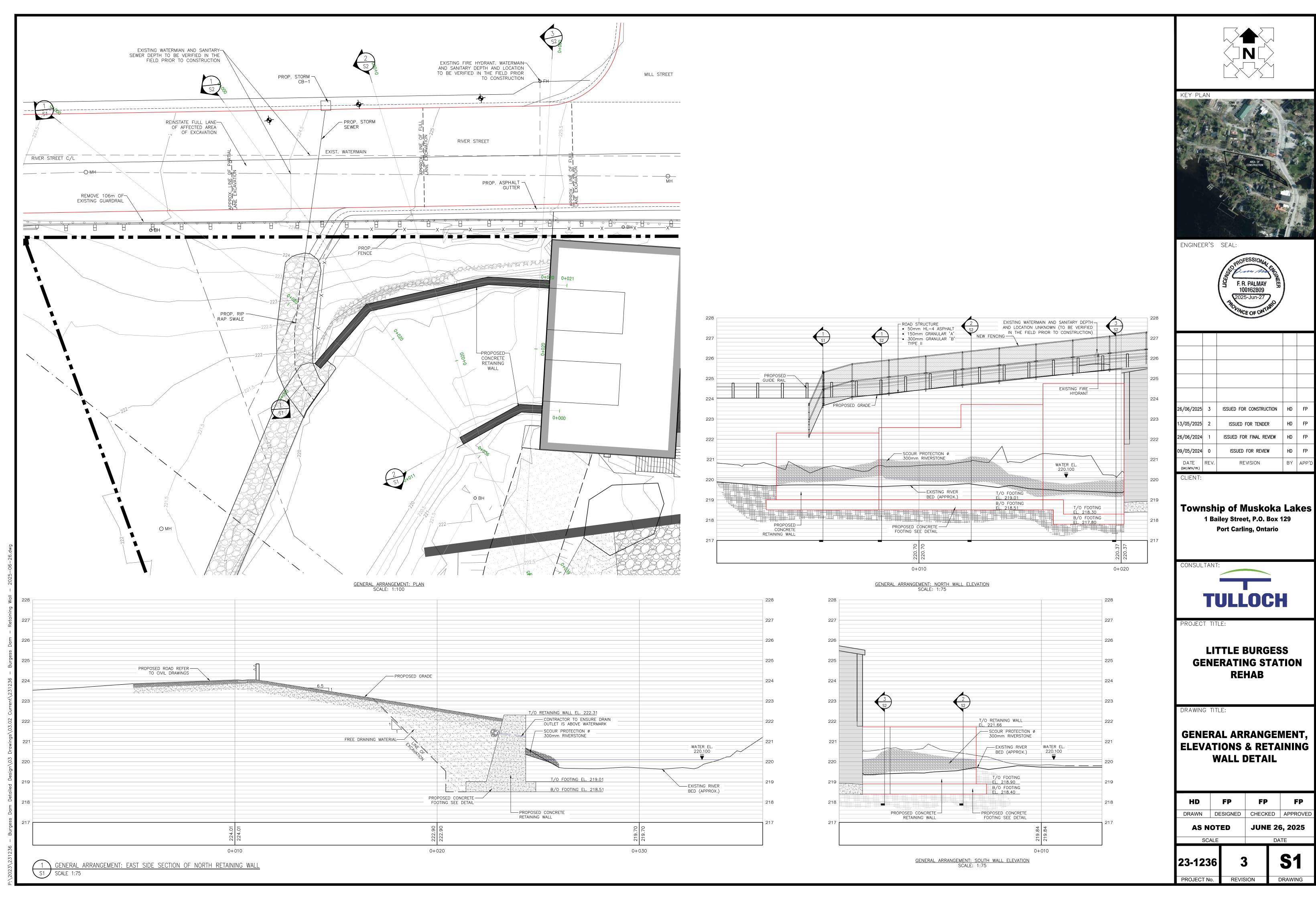
LITTLE BURGESS GENERATING STATION REHAB

DRAWING TITLE:

SEDIMENT & EROSION CONTROL PLAN

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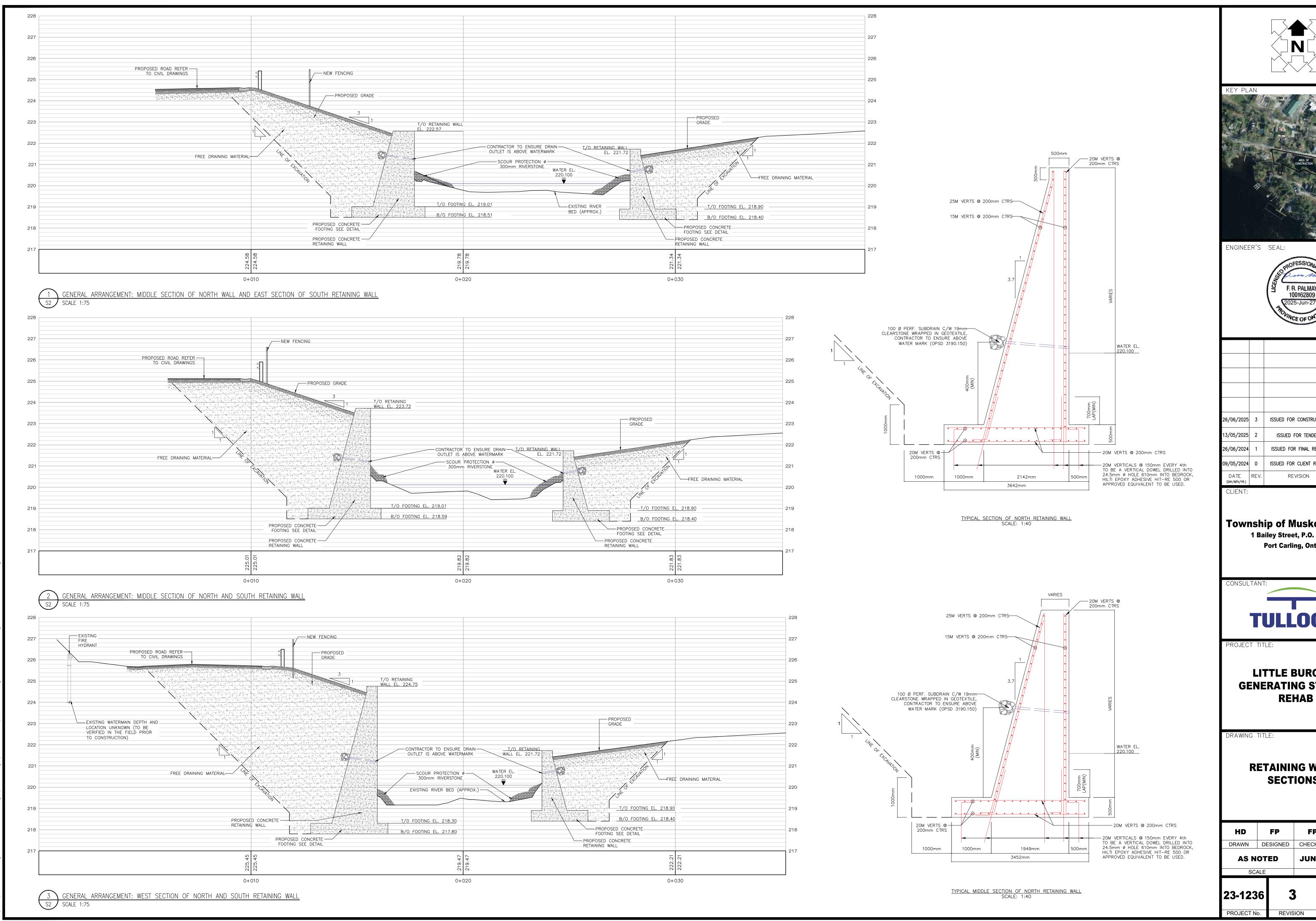
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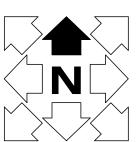




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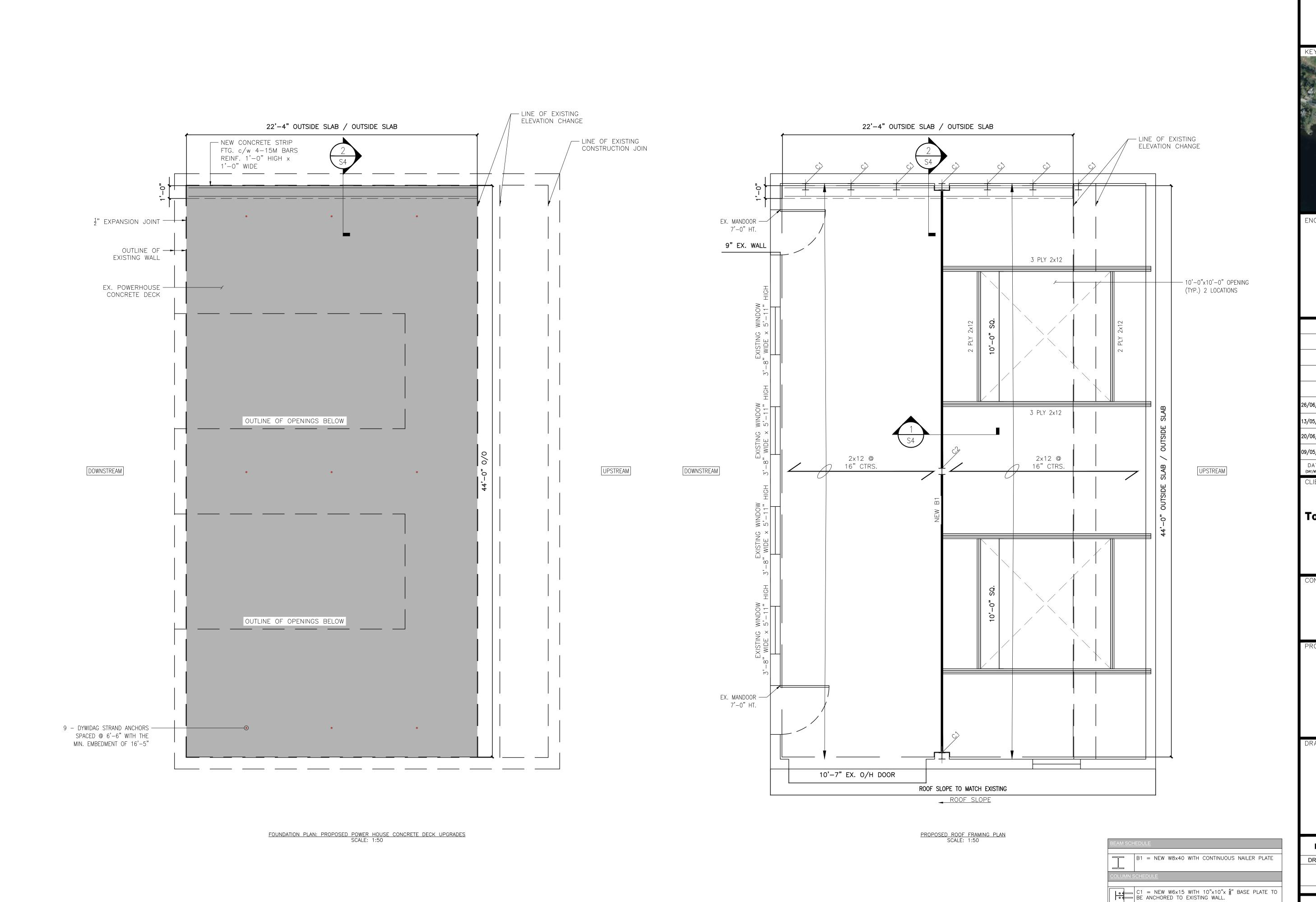
Township of Muskoka Lakes 1 Bailey Street, P.O. Box 129 **Port Carling, Ontario**

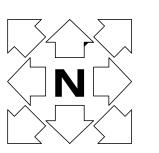
LITTLE BURGESS GENERATING STATION

RETAINING WALL SECTIONS

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ENGINEER'S SEAL:



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20/06/2024	1	ISSUED FOR REVIEW	HD	FP
09/05/2024	0	ISSUED FOR REVIEW	HD	FP
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CLIENT:				

Township of Muskoka Lakes 1 Bailey Street, P.O. Box 129 Port Carling, Ontario



PROJECT TITLE:

LITTLE BURGESS GENERATING STATION REHAB

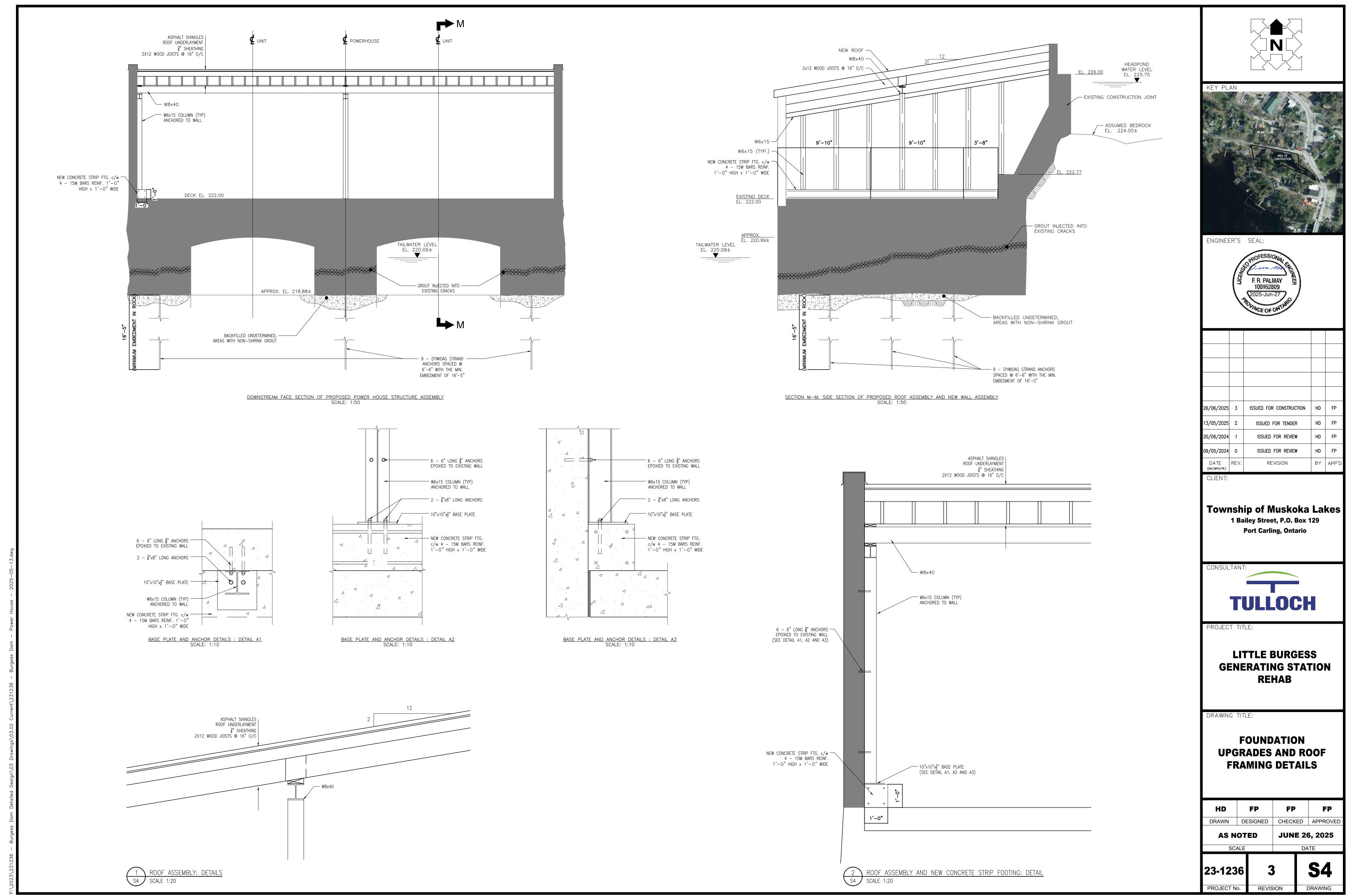
DRAWING TITLE:

C2 = NEW W6x15 WITH 10"x10"x §" BASE PLATE.

FOUNDATION UPGRADES & ROOF FRAMING PLANS

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S3 23-1236 PROJECT No. DRAWING REVISION





26/06/2025	3	ISSUED FOR CONSTRUCTION	HD	FP
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09/05/2024	0	ISSUED FOR REVIEW	HD	FP
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<u>PHOTO 1</u> DOWNSTREAM SIDE WEST ELEVATION LOOKING EAST

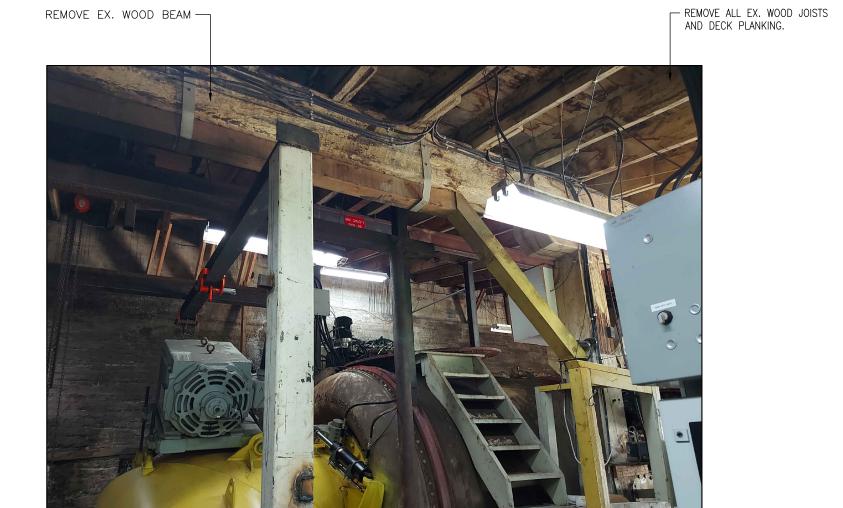
REMOVE EX. WOOD BEAM —



<u>PHOTO 2</u> EXISTING BEAM AND ROOF STRUCTURE REMOVE EX. WOOD — COLUMN AND BRACING



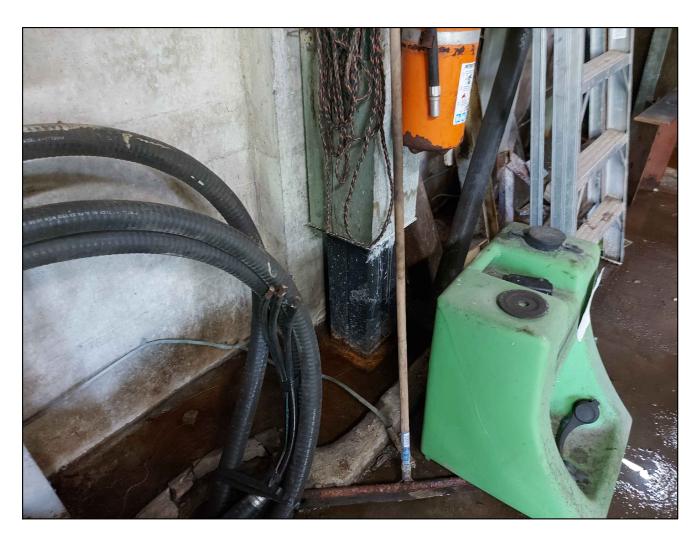
<u>PHOTO 3</u> EXISTING BEAM AND ROOF STRUCTURE



<u>PHOTO 4</u> EXISTING BEAM AND ROOF STRUCTURE



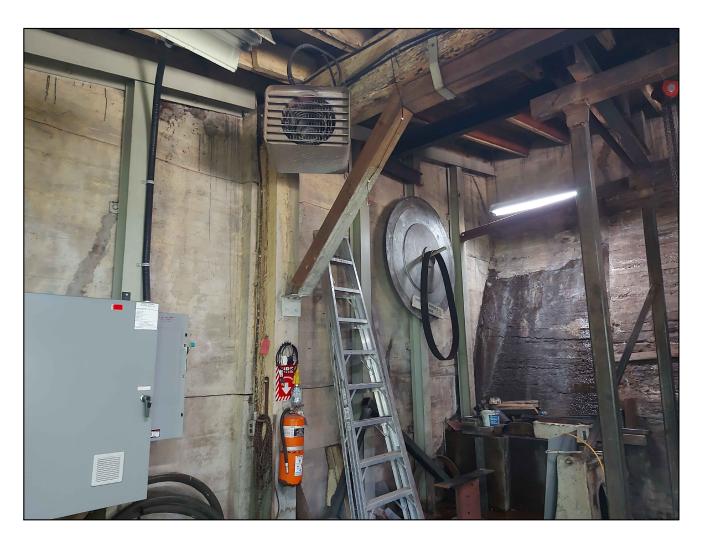
<u>PHOTO 5</u> EXISTING BEAM AND ROOF STRUCTURE



<u>PHOTO 6</u> EXISTING STEEL COLUMN BASE — NORTH WALL



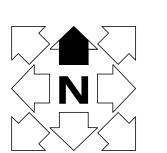
<u>PHOTO 7</u> EXISTING STEELWORK — NORTH WALL



<u>PHOTO 8</u> EXISTING STEELWORK — NORTH WALL

REMOVAL NOTES

- 1. REMOVE EXISTING WOOD FRAMED ROOF STRUCTURE AND REPLACE WITH NEW.
- 2. REMOVE EXISTING WOOD TIMBER CENTRE BEAM AND EX. WOOD COLUMNS AND REPLACE WITH NEW STEEL.
- 3. REMOVE ALL EXISTING STEEL COLUMNS ON NORTH WALL AND REPLACE WITH NEW.



- REMOVE EX. WOOD BEAM

ENGINEER'S SEAL: F. R. PALMAY

26/06/2025	3	ISSUED FOR CONSTRUCTION	HD	FP
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CLIENT:				

Township of Muskoka Lakes 1 Bailey Street, P.O. Box 129 **Port Carling, Ontario**



PROJECT TITLE:

LITTLE BURGESS GENERATING STATION REHAB

DRAWING TITLE:

REMOVAL PHOTOS

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<u>GENERAL NOTES (CIVIL):</u>

- DO NOT SCALE DRAWINGS.
- ALL STANDARDS ARE TO BE IN ACCORDANCE WITH CURRENT ONTARIO PROVINCIAL STANDARD DRAWINGS (OPSD), ONTARIO PROVINCIAL STANDARD SPECIFICATIONS (OPSS) AND ONTARIO BUILDING CODE (LATEST EDITION), UNLESS OTHERWISE NOTED.
- NOTIFY ALL UTILITIES AND MUNICIPAL AUTHORITIES 72 HOURS PRIOR TO COMMENCEMENT OF CONSTRUCTION. UTILITY PERSONNEL TO BE ON SITE WHEN EXCAVATING ADJACENT TO UNDERGROUND UTILITIES. SUPPORT UTILITIES IN ACCORDANCE WITH THE DIRECTIONS AND GUIDELINES OF THE IMPACTED UTILITY.
- THE LOCATION OF UTILITIES AND SERVICES SHOWN ON DRAWINGS IS APPROXIMATE AND MAY BE INCOMPLETE. CONFIRM EXACT LOCATION OF UTILITIES WITH MINISTRY, MUNICIPALITY OR UTILITIES. THE CONTRACTOR RESPONSIBLE TO DETERMINE LOCATIONS OF ALL UTILITIES PRIOR TO CONSTRUCTION AND WILL BE RESPONSIBLE FOR PROTECTING AGAINST DAMAGE. THE CONTRACTOR ASSUMES ALL LIABILITY FOR DAMAGE TO UTILITY AND
- COMPLY WITH THE REQUIREMENTS OF THE DISTRICT MUNICIPALITY OF MUSKOKA IN REGARD TO TRAFFIC FLOW ON MUNICIPAL STREETS. ALL NECESSARY TRAFFIC CONTROL AND SIGNAGE IS TO BE IN ACCORDANCE WITH MTO BOOK
- DESIGN DRAWINGS ARE TO BE READ IN CONJUNCTION WITH SPECIFICATIONS AND OTHER CONTRACT DOCUMENTS.
- ALL DIMENSIONS ARE IN METERS, EXCEPT PIPE DIAMETERS, WHICH ARE IN MILLIMETERS, UNLESS SPECIFIED
- THE CONTRACTOR IS RESPONSIBLE FOR ALL DETAILED LAYOUT WORK
- ALL SILTATION AND EROSION CONTROL MEASURES ARE TO BE IN PLACE PRIOR TO CONSTRUCTION AND SHALL REMAIN IN PLACE UNTIL THE DISTURBED AREAS ARE REINSTALLED TO THE EXISTING CONDITION IS BETTER.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL DE-WATERING AND WATER MANAGEMENT DURING CONSTRUCTION.
- LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION AND ALL DISCREPANCIES REPORTED TO THE
- TYPICAL DESIGN DETAILS SHOWN ON THE DRAWINGS SHALL GOVERN THE WORK. IF DETAILS ON OTHER DRAWINGS CONFLICT THE MOST STRINGENT SHALL GOVERN.
- ALL INSTALLATIONS AND CONSTRUCTION TO BE COMPLETED TO THE SATISFACTION OF THE ENGINEER AND THE TOWNSHIP OF MUSKOKA LAKES IN ACCORDANCE WITH THE CONTRACT DOCUMETNS
- COMPLETE ALL EXCAVATIONS AND FOUNDATION WORK IN ACCORDANCE WITH THE OCCUPATIONAL HEALTH & SAFETY ACT (LATEST EDITION).
- RESTORATION OF ALL DISTURBED AREAS SHALL BE 75mm TOPSOIL AND SEED MIX UNLESS NOTED OTHERWISE
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, DETAILS, AND ELEVATIONS OF THE EXISTING STRUCTURE THAT ARE RELEVANT TO THE WORK SHOWN ON THE DRAWING PRIOR TO THE COMMENCEMENT OF THE WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE CONTRACT ADMINISTRATOR AND THE PROPOSED ADJUSTMENT OF THE WORK REQUIRED TO MATCH THE EXISTING STRUCTURE SHALL BE SUBMITTED FOR APPROVAL.
- 7. CONTRACTOR ASSUMES ALL RESPONSIBILITY OF DESIGN, ERECTION AND REMOVAL OF TEMPORARY WORKS.
- 8. FORMWORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF CSA A23.1.
- DESIGN OF FORMWORK IS THE CONTRACTOR'S RESPONSIBILITY.
- O. CONTRACTOR TO PROVIDE TEMPORARY BRACING, SUPPORTS AND/OR OTHER INSTALLATIONS NECESSARY TO MAINTAIN PLUMBNESS, TRUE, ALIGNMENT AND STABILITY OF THE STRUCTURE AND ALL OF ITS PARTS
- IT IS THE ULTIMATE RESPONSIBILITY OF THE CONTRACTOR/OWNER TO ENSURE THAT ALL REQUIRED PERMITS AND APPROVALS ARE OBTAINED & POSTED PRIOR TO PROCEEDING WITH CONSTRUCTION. THESE PERMITS/APPROVALS MAY INCLUDE (BUT ARE NOT NECESSARILY LIMITED TO):
 - MUNICIPAL BUILDING DEPARTMENT PERMIT TO CONSTRUCT (MULTIPLE PERMITS MAY BE REQUIRED)
 - GOVERNMENTAL REGULATORS (CDA/MNRF)
 - ELECTRICAL SAFETY AUTHORITY (ESA) PERMIT

FOUNDATION PREPARATION:

THIS SPECIFICATION COVERS WORK ITEMS REQUIRED FOR CLEARING/ GRUBBING, STRIPPING, EXCAVATION AND FOUNDATION PREPARATION PRIOR TO PLACEMENT OF EMBANKMENT FILL MATERIALS AND CONSTRUCTION OF THE RELATED APPURTENANT STRUCTURES INCLUDING SPILLWAY, ACCESS ROADS, LAYDOWN AREA AND TEMPORARY

NATER MANAGEMENT

- THE EXISTING DAM CAN BE THE UPSTREAM COFFERDAM DURING REHABILITATION CONSTRUCTION. THE FOUNDATION PREPARATION FOR THE DOWNSTREAM WORK AREA WILL RESULT IN SOME SEEPAGE FROM UPSTREAM (THE EXISTING DAM FOUNDATION). THE SEEPAGE RATE IS CONSIDERED TO BE MANAGEABLE BY SUMPS AND PUMPS DURING CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY FOR WATER MANAGEMENT DURING CONSTRUCTION.
- SOME SEEPAGE AND /OR RUN-OFF WATER MAYBE ENCOUNTERED FROM SURROUND AREAS AT THE SITE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTROL OF RUN-OFF STORM EVENT DURING CONSTRUCTION.
- 3. THE CONTRACTOR'S PROPOSED WATER MANAGEMENT PLAN SHALL BE SUBMITTED TO THE CLIENT'S APPROVAL.

FOUNDATION PREPARATION:

- THE WORK TO BE DONE UNDER THIS ITEM COMPRISES THE SUPPLY OF ALL LABOUR, PLANT AND MATERIAL, AND THE PERFORMANCE OF ALL WORK NECESSARY FOR CLEARING AND GRUBBING THE CONSTRUCTION AREA, AS SHOWN
- CLEARING AND GRUBBING SHALL CONSIST OF CUTTING AND DISPOSING OF ALL TREES, HEDGES, SHRUBS ALIVE OR DEAD, DEBRIS AND ALL OTHER PERISHABLE MATERIALS, INCLUDING FALLEN TREES AND LOGS WHICH MAY BE VISIBLE ON THE SURFACE OF THE GROUND WITHIN THE CONSTRUCTION AREA. ALL TREES, HEDGES, AND SHRU SHALL BE CUT OFF AT THE NATURAL GROUND SURFACE IN ALL AREAS OF THE CONSTRUCTION AS SHOWN ON THE
- THE CONTRACTOR SHALL GIVE THE CLIENT OR ITS REPRESENTATIVE AT LEAST THREE (3) DAYS NOTICE OF INTENT TO CLEAR AREAS. THE LIMITS OF CLEARING AS APPROVED BY THE CLIENT OR ITS REPRÉSENTATIVE AND AS SPECIFIED HEREIN SHALL BE STRICTLY ADHERED TO.
- . REMOVE ALL CLEARED MATERIAL FROM THE SITE AND DISPOSED OF IN A MANNER ACCEPTABLE TO THE CLIENT OR
- THE WORK SHALL BE CARRIED OUT STRICTLY IN ACCORDANCE WITH THE ENVIRONMENTAL ASSESSMENT PERMIT ISSUED FOR THE PROJECT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING AND COMPLYING WITH ANY AND ALL PERMITS ASSOCIATE WITH CLEARING, GRUBBING AND PERMANENT DISPOSAL OF MATERIALS.
- 5. CLEARING, GRUBBING AND STRIPPING SHALL BE COMPLETED PRIOR TO THE START OF EARTHWORKS AND SHALL BE AT A MINIMUM DISTANCE OF FIVE (5) METERS FROM FILL PLACEMENT. CONTRACTOR CAN ONLY PROCEED WITH FILL PLACEMENT AFTER RECEIVING WRITTEN APPROVAL FROM THE CLIENT OR ITS REPRESENTATIVE.
- STRIPPING, EXCAVATION AND STOCKPILING:
- THE WORK SHALL COMPRISE THE SUPPLY OF ALL LABOUR AND PLANT, AND THE PERFORMANCE OF ALL WORK NECESSARY FOR THE STRIPPING, EXCAVATION AND STOCKPILING FOR SUBSEQUENT REUSE OR DISPOSING OF STRIPPED AND EXCAVATED MATERIALS FROM THE CONSTRUCTION AREA OF THE DAM, ACCESS ROAD AND LAYDOWN
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE WITH THE CLIENT FOR CLEARANCE OF ANY BURIED UTILITIES, EXITING GROUND GRID AND INSTRUMENTATION AT THE DAM SITE. BURIED UTILITIES, GROUNDING GRID AND INSTRUMENTATION SHALL BE PROTECTED OR REMOVED WITHOUT DAMAGE BEFORE STRIPPING AND EXCAVATION ACTIVITY. NO STRIPPING OF ANY AREA SHALL START WITHOUT PRIOR WRITTEN APPROVAL FROM THE CLIENT OR ITS REPRESENTATIVE.
- THE STRIPPING AND EXCAVATED MATERIALS AT THE EXISTING DAM SITE SHALL NOT BE REUSED FOR ENGINEERED DAM FILL MATERIALS. THE MATERIAL MAY NEED TO BE DISCARDED AT THE CLIENT OR ITS REPRESENTATIVE'S DISCRETION. THE STRIPPED MATERIAL NOT SUITABLE FOR RE-USE SHALL BE DISPOSED OF BY THE CONTRACTOR
- OFF SITE AND DISPOSED OF IN A MANNER ACCEPTABLE TO THE CLIENT OR ITS REPRESENTATIVE. THE EXCAVATED MATERIALS FROM THE EXISTING DAM SITE AS PER DESIGN SHALL BE TEMPORARILY STOCKPILED AT

GRANULAR FILL PLACEMENT AND COMPACTION:

1. THIS SPECIFICATION DESCRIBES THE REQUIREMENTS FOR PLACEMENT AND COMPACTION OF GRANULAR FILL MATERIALS INCLUDING ROCKFILL/RIP RAP FOR EMBANKMENT DAM BUTTRESS, SPILLWAY, AND CREEK BANK

MATERIAL CODES AND TEST STANDARDS:

2. THE GRANULAR FILL MATERIALS SHALL FOLLOW THE ASTM STANDARDS INCLUDING:

<u>ASTM</u> <u>DESCRIPTION</u> ASTM D75 METHODS FOR SAMPLING AGGREGATES

METHOD FOR PARTICLE SIZE ANALYSIS

EROSION PROTECTION CONSTRUCTION AS PER DESIGN DRAWINGS.

- TEST METHOD FOR MOISTURE-DENSITY RELATIONS OF SOILS AND SOIL AGGREGATE MIXTURES USING 4.4LB. (2.5 KG) RAMMER AND 305mm (12") DROP
- ASTM D854
- TEST METHOD FOR SPECIFIC GRAVITY OF SOILS TEST METHOD FOR AMOUNT OF MATERIALS IN SOILS FINER THAN THE NO. 200 SIEVE
- SOIL-AGGREGATE MIXTURES ASTM D2922 TEST METHODS FOR DENSITY OF SOIL AND SOIL-AGGREGATE IN PLACE BY NUCLEAR

METHOD FOR LABORATORY DETERMINATION OF WATER (MOISTURE) OF SOIL, ROCK AND

- ASTM D3017 TEST METHODS FOR MOISTURE CONTENT OF SOIL AND SOIL-AGGREGATE IN PLACE BY
- STANDARD TEST METHOD FOR PARTICLE SIZE ANALYSIS OF NATURAL AND MAN-MADE RIP RAP MATERIALS

NUCLEAR METHODS (SHALLOW DEPTH)

WEIGHT) OF SOIL SPECIMENS

ASTM D2216

ASTM D7263

DIMENSIONS SHALL NOT BE LESS THAN THOSE SPECIFIED AND FINAL SLOPES SHALL NOT BE STEEPER THAN THOSE INDICATED ON THE DESIGN DRAWINGS. UNLESS OTHERWISE SHOWN ON THE DRAWINGS, THE ACCEPTABLE DIMENSIONAL TOLERANCES FOR THE FILL PLACEMENT SHALL BE THE FOLLOWING:

STANDARD TEST METHODS FOR LABORATORY DETERMINATION OF DENSITY (UNIT

- LEVEL TOLERANCE: +/- 0.1m HORIZONTAL TOLERANCE +/- 0.3m
- SHOULD ANY ERRORS IN SETTING OUT THE WORKS OCCUR, SUCH ERRORS SHALL BE CORRECTED AND ANY NECESSARY ADJUSTMENT TO PREVIOUSLY PLACED FILL MATERIALS RESULTING FROM SUCH ERRORS SHALL BE MADE GOOD TO THE SATISFACTION OF THE ENGINEER AT THE SITE PRIOR TO FURTHER PLACEMENT OF FILL MATERIALS AT NO COST TO THE CLIENT.

- 1. THE GRANULAR FILL MATERIALS FOR THE WORK AS PER DESIGN DRAWING SHALL BE:
- FREE DRAINING GRANULAR FILL AS NOTED ON DESIGN DRAWINGS

MATERIAL SHALL NOT BE USED FOR GRANULAR FILL MATERIALS.

- 2. THE GRANULAR FILL MATERIALS SHALL CONSIST OF NATURALLY OCCURRING (AS SPECIFIED HERE), OR CRUSHED, CLEAN, SOUND, MINERAL PARTICLES, NON-PLASTIC, FREE FROM ROOTS AND TOPSOIL OR OTHER DEBRIS MATERIALS CONTAINING QUANTITIES OF ORGANIC MATTER, FLAT OR ELONGATED PARTICLES, DELETERIOUS
- MATERIAL PLACED WHICH DOES NOT MEET THE SPECIFIED GRADATION REQUIREMENTS SHALL BE REMOVED, BLENDED OR OTHERWISE REWORKED TO PRODUCE A MATERIAL WHICH DOES.
- OVERSIZED PARTICLES SHALL BE REMOVED, EITHER AT THE SOURCE OR DURING PLACEMENT OR BOTH. WHERE REDUCED LIFT THICKNESSES ARE REQUIRED, PARTICLES LARGER THAN 2/3 OF THE REDUCED LIFT THICKNESS
- SHALL BE REMOVED PRIOR TO COMPACTION. 5. THE GRANULAR FILL MATERIALS SHOULD BE NON-METAL LEACHING AND ARD POTENTIAL MATERIALS.
- ROCKFILL

ROCKFILL MATERIAL FOR EMBANKMENT FILL SHALL BE FROM CRUSHED ROCK SPALLS IN ACCORDANCE WITH THE GEOTEXTILE:

SIEVE SIZES (US STANDARD SIZE) % PASSING GRAIN SIZE (mm)

1-1/2"

FOLLOWING GRADATIONS:

- SPILLWAY RIP RAP RIP RAP MATERIAL FOR EMBANKMENT AND SPILLWAY EROSION PROTECTION SHALL BE WELL GRADED SIZES OF HARD STRONG DURABLE, DENSE, AND HIGH-QUALITY ROCK FRAGMENTS. THE FRAGMENTS SHALL BE GENERALL CUBIC IN SHAPE WITH THE LARGEST DIMENSION NOT MORE THAN TWICE THE SMALLEST. THE RIP RAP SHALL MEET
- MAXIMUM SIZE (D_{MAX}): 750 mm (±50mm) MEDIAN SIZE (D₅₀): 370 mm
- MINIMUM SIZE (D_{MIN}): 180 mm (±50mm) GRANULAR FILL PLACEMENT AND COMPACTION:
- 1. THE WORK TO BE DONE SHALL COMPRISE THE SUPPLY OF ALL LABOUR, PLANT AND MATERIAL, AND THE ERFORMANCE OF ALL WORK NECESSARY FOR SUPPLYING, PLACING AND COMPACTING GRANULAR FILL MATERIALS, AS SHOWN ON THE DESIGN DRAWINGS.
- 2. GRANULAR FILL MATERIALS SHALL BE PLACED TO THE LINES AND GRADES SHOWN ON THE DRAWINGS AND AS REQUIRED BY THE ENGINEER BASED ON SPECIFIC SITE CONDITIONS.
- 3. PLACING AND SPREADING OF GRANULAR FILL MATERIAL IN ACCORDANCE WITH THESE SPECIFICATIONS SHALL BE PERFORMED IN SUCH A MANNER AS TO AVOID SEGREGATION OF SIZES AND TO OBTAIN A HOMOGENEOUS MASS.
- THE SELECTION OF COMPACTION EQUIPMENT SHALL BE SUBJECT TO THE APPROVAL OF ENGINEER AND TO CONTINUING SATISFACTORY PERFORMANCE.

NATIVE MATERIAL, SUITABLE FOR BACKFILL, SHALL BE COMPACTED TO 98% STANDARD PROCTOR MAXIMUM DRY

- GRANULAR MATERIAL USED FOR BACKFILL AND EMBEDMENT SHALL BE PLACED IN LAYERS 200mm IN DEPTH
- MAXIMUM AND COMPACTED TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY UNLESS OTHERWISE NOTED.
- WHERE NECESSARY TO ACHIEVE THE SPECIFIED COMPACTION, WATER SHALL BE APPLIED BY CONTROLLED SPRINKLING TO THE GRANULAR FILL MATERIALS IN AMOUNTS AS APPROVED BY THE ENGINEER. 8. THE STOCKPILING OF GRANULAR FILL MATERIAL WILL BE INSPECTED AND APPROVED BY ENGINEER, AT APPROVED
- LOCATIONS, PROVIDED THAT CONTRACTOR EXERCISES EVERY PRECAUTION NECESSARY AND BEST PRACTICES TO PREVENT SEGREGATION OF PARTICLE SIZES. THE STOCKPILED MATERIAL SHALL MEET THE GRADATION GRANULAR FILL MATERIAL SHALL NOT BE CONTAMINATED BY MIXING WITH OTHER MATERIALS. FILL MATERIALS
- WHICH HAVE BECOME CONTAMINATED SHALL BE REMOVED AND REPLACED AT NO ADDITIONAL COST TO THE
- . GRANULAR FILL MATERIALS SHALL BE PLACED, SPREAD AND COMPACTED IN A DIRECTION PARALLEL TO THE AXIS OF THE DAM EXCEPT AS OTHERWISE PERMITTED BY ENGINEER BASED ON THE SPECIFIC SITE CONDITION. 11. THE CONTRACTOR MAY USE AN ALTERNATIVE TYPE OF ROLLER OR EQUIPMENT FOR COMPACTION PROVIDED THAT
- THE REQUIREMENT OF THE SPECIFICATION. 12. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE QUALITY OF WORK IN ACCORDANCE WITH THESE SPECIFICATIONS. ENGINEER WILL CARRY OUT QUALITY CONTROL TESTING OF CONTRACTOR'S OPERATIONS, AND CONTRACTOR SHALL COOPERATE IN EVERY WAY TO ENSURE THAT TESTS CAN BE PERFORMED AT ALL LOCATIONS

 2. PHYSICAL PROPERTIES OF THE GEOTEXTILE SHALL INCLUDE: AND TIMES AS REQUIRED BY THE ENGINEER. THE CONTRACTOR SHALL VARY THE METHOD OF PLACING AND COMPACTION IN ORDER TO MEET THE REQUIREMENTS OF THESE SPECIFICATIONS AS DETERMINED BY TESTING AND

THE PERFORMANCE OF SUCH EQUIPMENT IS DEMONSTRATED TO PRODUCE THE COMPACTION SPECIFIED TO MEET

- AS ACCEPTED BY THE ENGINEER. . THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THE SOURCE OF ALL THE REQUIRED GRANULAR FILL MATERIALS SPECIFIED IN THE DESIGN DRAWING AND ASSOCIATE TESTING DATA RESULTS WITHIN 7 DAYS OF AWARD OF CONTRACT. ADDITIONAL TESTING OF THE MATERIAL MAY BE REQUIRED BY THE ENGINEER DURING THE WORK TO DEMONSTRATE CONFORMANCE TO THE SPECIFIED GRADATIONS.
- 1. EACH LAYER OF ROCKFILL MATERIAL SHALL BE COMPACTED BY NOT LESS THAN FOUR COMPLETE PASSES OF THE VIBRATORY ROLLER EQUIPMENT. VIBRATORY ROLLER SHALL HAVE A SMOOTH STEEL DRUM, EQUIPPED WITH CLEANING DEVICES, EITHER PULLED OR SELF-PROPELLED. COMPACTOR SHALL HAVE A STATIC WEIGHT OF 1 TONNES ON THE AXIS OF THE DRUM. THE VIBRATION FREQUENCY OF THE VIBRATORY ROLLER DURING OPERATION SHALL BE BETWEEN 20HZ AND 35HZ AND THE COMPACTION (CENTRIFUGAL) FORCE DEVELOPED BY THE ROLLER AT 30 HZ SHALL NOT BE LESS THAN 19 T. THE POWER OF THE ENGINE DRIVING THE VIBRATORY ROLLER SHALL BE SUFFICIENT TO MAINTAIN THE SPECIFIED FREQUENCY AND CENTRIFUGAL FORCE UNDER THE MOST ADVERSE CONDITIONS WHICH MAY BE ENCOUNTERED DURING COMPACTION OF THE FILL
- 2. THE VIBRATORY ROLLER SHALL MAINTAIN A MINIMUM DISTANCE OF 1.5m FROM THE EXISTING CONCRETE DAM FOR COMPACTION. ROCKFILL MATERIAL AROUND THE EXISTING CONCRETE DAM SHALL BE LAID IN 300mm LAYERS AND SHALL BE COMPACTED WITH HAND COMPACTORS TO 98% SPMDD.
- 3. ROCKFILL MATERIAL UNDER WATER SHALL BE PLACED WITH MECHANICAL EQUIPMENT AND COMPACTED UNDER WATER WITH THE BACK OF EXCAVATOR BUCKET SO THAT NO DEFORMATION CAN BE OBSERVED. EACH LAYER OF ROCKFILL MATERIAL UNDER WATER SHALL NOT BE MORE THAN 250mm THICK BEFORE COMPACTION. 4. IF REQUIRED BY THE ENGINEER, CONTRACTOR SHALL VARY THE NUMBER OF PASSES OF COMPACTION EQUIPMENT 3. MATERIAL ACCEPTANCE SHALL ADHERE TO ASTM D 4759. FROM THE MINIMUM FOUR COMPLETE PASSES SPECIFIED, IN ORDER TO ACHIEVE THE SPECIFIED DENSITY.
- 1. RIP RAP SHALL BE PLACED BY SUITABLE MECHANICAL EQUIPMENT SO THAT INTERMIXING OF ADJOINING ROCKFILL AND RIP RAP DOES NOT OCCUR DURING PLACING AND SO THAT MINIMUM BREAKAGE OF ROCK FRAGMENTS OCCURS.
- 2. RIP RAP NEED NOT BE COMPACTED BUT PLACED CAREFULLY IN SUCH A MANNER THAT LARGER ROCK FRAGMENTS ARE UNIFORMLY DISTRIBUTED AND SMALLER ROCK FRAGMENTS FILL VOIDS BETWEEN LARGER PIECES. HAND PLACING SHALL BE REQUIRED TO THE EXTENT NECESSARY TO SECURE RESULTS SPECIFIED ABOVE. THE SPECIFIED ROCK SIZES AND GRADING APPLY TO RIP RAP IN-PLACE.

CONCRETE CAST-IN-PLACE AND REPAIR:

- THE CONCRETE CAST-IN-PLACE AND REPAIR WORK FOR THIS PROJECT MAINLY CONSISTS OF: 1. REMOVING THE CONCRETE STRUCTURE AS SHOWN AND DESCRIBED ON THE DRAWINGS AND IN THIS SPECIFICATION.
- 2. PREPARING THE SURFACE FOR THE CONCRETE CAST-IN-PLACE AS SHOWN ON THE DRAWINGS, INCLUDING
- ABRASIVE CLEANING, CLEANING OF EXISTING REINFORCEMENT, AND APPLYING BONDING AGENT TO THE SURFACE.
- 3. SUPPLYING MATERIALS AND THE MIXING AND PLACING OF CONCRETE AS SHOWN AND DESCRIBED ON THE DRAWINGS AND IN THIS SPECIFICATION INCLUDING VIBRATING, FINISHING, AND CURING.
- 4. SUPPLYING, FABRICATING, CONSTRUCTING, MAINTAINING AND REMOVING TEMPORARY WORKS, INCLUDING FALSEWORK
- 5. THE QUALITY CONTROL (QC) TESTING OF ALL CONCRETE MIX MATERIALS.

REFERENCES AND STANDARDS:

- 1. CSA A23.1/A23.2, CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION/METHODS OF TEST AND STANDARD PRACTICES FOR CONCRETE.
- 2. CAN/CSA A3001, CEMENTITIOUS MATERIALS FOR USE IN CONCRETE.
- 3. INTERNATIONAL CONCRETE REPAIR INSTITUTE (ICRI) TECHNICAL GUIDELINE NO. 310.1R, GUIDE FOR SURFACE.
- 4. PREPARATION FOR THE REPAIR OF DETERIORATED CONCRETE RESULTING FROM REINFORCING STEEL CORROSION.
- 5. CAN/CSA G30.18, BILLET-STEEL BARS FOR CONCRETE REINFORCEMENT.
- 7. REINFORCING STEEL INSTITUTE OF CANADA, (RSIC), MANUAL OF STANDARD PRACTICE.

6. ACI 117, STANDARD TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS.

8. GUIDE TO CONCRETE REPAIR (2ND EDITION). U.S. DEPARTMENT OF THE INTERIOR BUREAU OF RECLAMATION.

PROCESSING.

- **CONCRETE REMOVAL:** 1. ALL AREAS OF THE EXISTING CONCRETE TO BE REMOVED WILL BE MARKED BY THE CONTRACTOR AS SHOWN ON DRAWINGS. THE CONCRETE REMOVAL AREAS SHALL BE CONFIRMED AND APPROVED BY THE ENGINEER BEFORE
- 2. THE CONTRACTOR SHALL REMOVE THE EXISTING CONCRETE AS THE DIMENSIONS SHOWN ON THE DRAWINGS OR AS DIRECTED BY THE ENGINEER. ANY DAMAGE CAUSED BY THE CONTRACTOR TO ANY PORTION OF THE STRUCTURE NOT INTENDED FOR THE REMOVAL SHALL BE REPAIRED/RECOVERED BY THE CONTRACTOR, AT THE CONTRACTOR'S EXPENSE. TO THE SATISFACTION OF THE ENGINEER.
- 3. THE CONTRACTOR SHALL EXERCISE CAUTION AND TAKE CARE NOT TO DAMAGE ANY EXISTING DAM. POWERHOUSE. ROAD, AND EARTH RETAINING STRUCTURE, INTENDED TO REMAIN IN PLACE. CONCRETE CAST-IN-PLACE:
- 1. CONCRETE MIX DESIGN SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
- MINIMUM 28-DAY COMPRESSIVE STRENGTH 30 MPA, CLASS F1. • CEMENTITIOUS MATERIAL: TYPE GU WITH UP TO 30% FLY ASH.
- MAXIMUM WATER/CEMENT RATIO BY WEIGHT: 0.45
- SLUMP: 75mm +- 25mm (BEFORE ADDITION OF SUPERPLASTICIZER).
- AIR CONTENT AS PER CSA A23.1

AGGREGATE: 20mm MAX. NOMINAL SIZE.

- EXECUTION ACCORDING TO CSA A23.1, LATEST EDITION.
- CURING ACCORDING TO CSA A23.1, LATEST EDITION.
- 2. ALL CONCRETE SHALL MEET THE REQUIREMENTS OF CSA A23.1 AND A23.2 STANDARDS, LATEST EDITION.
- 3. AGGREGATES SHALL BE TESTED FOR ALKALI REACTIVITY AS PER CSA A23.1 AND A23.2 AND APPROVED BY THE
- 4. BACK-FILLING AGAINST CAST-IN-PLACE CONCRETE WALLS SHALL NOT COMMENCE UNTIL CONCRETE IN THE WALL HAS ACHIEVED DESIGN STRENGTH.

- PROVIDE EROSION CONTROL FOR OVERFLOW ROCKFILL/RIP RAP FILL OF DOWNSTREAM SPILLWAY CHANNEL AND/OR
- THE AREA SUBJECT TO FLOW/RUN OFF EROSION AND OR SCOUR DURING DAM OPERATION. • INTERFACE BETWEEN THE ROCKFILL/RIP RAP AND EXCAVATED NATIVE SOIL FOR THE CREEK BANK EROSION • SEPARATE AND PREVENT MIXING OF MATERIALS OF DIFFERENT GRADING AS REQUIRED IN THE DESIGN DRAWING
- APPLICABLE CODES AND STANDARDS: THE FOLLOWING CODES AND STANDARDS APPLY FOR THE USE OF GEOTEXTILES:
- ASTM D 4354 PRACTICE FOR SAMPLING OF GEOSYNTHETICS FOR TESTING 2. ASTM D 4355 TEST METHOD FOR DETERIORATION OF GEOTEXTILES FROM EXPOSURE TO ULTRAVIOLET LIGHT AND
- WATER (XENON-ARC TYPE APPARATUS) 3. ASTM D 4491-99A, STANDARD TEST METHODS FOR WATER PERMEABILITY FOR GEOTEXTILES BY PERMITTIVITY
- 4. ASTM D 4595-86 (2001), STANDARD TEST METHOD FOR TENSILE PROPERTIES OF GEOTEXTILES BY THE WIDE-WIDTH STRIP METHOD.
- 5. ASTM D 4632 TEST METHOD FOR GRAB BREAKING LOAD AND ELONGATION OF GEOTEXTILES. 6. ASTM 4716-01, TEST METHOD FOR DETERMINING THE (IN-PLANE) FLOW RATE PER UNIT WIDTH AND HYDRAULIC
- TRANSMISSIVITY OF A GEOSYNTHETIC USING A CONSTANT HEAD. 7. ASTM D 4751-99A, STANDARD TEST METHOD FOR DETERMINING APPARENT OPENING SIZE OF A GEOTEXTILE.
- 9. ASTM D4833, TEST METHOD FOR INDEX PUNCTURE RESISTANCE OF GEOTEXTILES, GEOMEMBRANES AND RELATED
- 10. ASTM D4873, GUIDE FOR IDENTIFICATION, STORAGE, AND HANDLING OF GEOTEXTILES.

8. ASTM D4759, PRACTICE FOR DETERMINING THE SPECIFICATION CONFORMANCE OF GEOSYNTHETICS.

- 11. ASTM D4884, STANDARD TEST METHOD FOR STRENGTH OF SEWN OR THERMALLY BONDED SEAMS OF GEOTEXTILES.
- 12. ASTM D5261, TEST METHOD FOR MEASURING MASS PER UNIT AREA OF GEOTEXTILES. 13. ASTM D5321, STANDARD TEST METHOD FOR DETERMINING THE COEFFICIENT OF SOIL AND GEOSYNTHETIC OR GEOSYNTHETIC AND GEOSYNTHETIC FRICTION BY THE DIRECT SHEAR METHOD.
- SHIPPING, STORAGE, AND HANDLING: 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHIPPING. STORAGE. AND HANDLING OF GEOTEXTILE. AS PER THE ASTM, AND AS WELL AS ANY RELATED COSTS, INCIDENTAL OR OTHERWISE, INCLUDED IN THE INSTALLATION

14. GRI STANDARD GT12(A) (EDITORIAL CHANGES), GEOSYNTHETIC RESEARCH INSTITUTE. DECEMBER 19, 2012.

- 2. THE GEOTEXTILE SHALL ADHERE TO THE LABELLING REQUIREMENTS OF ASTM D 4873 AND SHIPPING, HANDLING AND STORAGE SHALL ADHERE TO ASTM D 4873.
- MATERIAL TYPE: 1. THE GEOTEXTILE SHALL BE OF NON-WOVEN SYNTHETIC FIBER, SUPPLIED IN ROLLS:
- MINIMUM WIDTH OF 4.6m;
- MINIMUM LENGTH OF 91m; AND • COMPOSED OF MINIMUM 85% BY MASS OF POLYPROPYLENE WITH INHIBITORS ADDED TO BASE PLASTIC TO RESIST

DETERIORATION BY ULTRA-VIOLET AND HEAT EXPOSURE FOR 60 DAYS.

PHYSICAL PROPERTY	<u>VALUE</u>	<u>UNIT</u>	<u>ASTM</u>
NOMINAL THICKNESS	4.1	mm	ASTM D 5199
MASS PER UNIT AREA	540	G/m2	ASTM D 5261
TENSILE STRENGTH	1.64	KN	ASTM D 4632
ELONGATION AT BREAK	50%	_	ASTM D 4632
TRAPEZOIDAL TEAR STRENGTH	0.64	KN	ASTM D 4533
PUNCTURE STRENGTH	1.0	KN	ASTM D 4833
MINIMUM MULLEN BURST	5170	KPA	_
UV RESISTANCE	MINIMUM 70%	_	ASTM D4355
APPARENT OPENING SIZE	0.2	mm	ASTM D 4751
MINIMUM WATER FLOW RATE	2,000	L/MIN/m ²	ASTM D 4491
PERMITTIVITY	0.7	S ⁻¹	ASTM D 4491

PLACEMENT OF MATERIAL LAYERS.

- 1. PLACE GEOTEXTILE ON APPROVED SUBGRADE, FOLLOWING THE REQUIRED EXCAVATION AND STRIPPING, SITE
- PREPARATION FOR GEOTEXTILE INSTALLATION AND SMOOTH-ROLLING THE PREPARED SUBGRADE SURFACE. 2. PLACE GEOTEXTILE SMOOTH AND FREE OF TENSION STRESS, FOLDS, WRINKLES AND CREASES.

3. GEOTEXTILE PLACED ON SLOPES SHALL BE STARTED FROM THE TOE TO THE UPPER EXTENT OF THE SLOPE.

6. PROTECT PLACED GEOTEXTILE FROM DISPLACEMENT, DAMAGE OR DETERIORATION BEFORE, DURING AND AFTER

4. THE GEOTEXTILE SHALL BE JOINED BY OVERLAPS, THE MINIMUM OVERLAP BEING 500mm. 5. ANY DAMAGED GEOTEXTILE SHALL BE PROMPTLY REPAIRED OR REPLACED TO THE APPROVAL OF THE ENGINEER

STRUCTURAL STEEL:

1. STRUCTURAL STEEL SHALL CONFORM TO CAN/CSA-S19.

TIGHT CONDITION AS DEFINED BY CAN/CSA S16.1.

- 2. STRUCTURAL STEEL SHALL BE NEW G40.21 350W MATERIAL UNLESS NOTED OTHERWISE.
- 3. HOLLOW STRUCTURAL SECTIONS SHALL BE ASTM A500 GRADE C OR G40.21 350W MATERIAL UNLESS NOTED OTHERWISE
- 4. ANGLES CHANNELS AND PLATES TO BE G40-.21-300W MATERIAL UNLESS NOTED OTHERWISE.
- 5. WELDING TO CSA W59 WELDED STEEL CONSTRUCTION, LATEST EDITION
- 6. FABRICATOR SHALL BE CERTIFIED ACCORDING TO CSA STANDARD W47.1 DIVISION 1 OR 2, LATEST MIN. COMPRESSIVE STRENGTH
- 8. MEMBERS SHALL BE CONNECTED FOR THE FORCES INDICATED. WHERE THESE FORCES ARE NOT SHOWN, COMPRESSION AND TENSION SHOULD BE CONNECTED FOR 50% OF THE EFFECTIVE STRENGTH OF THAT MEMBER AND BEAMS CONNECTED FOR THE FULL UNIFORM CAPACITY FOR
- 9. SPLICES SHALL BE DESIGNED TO DEVELOP THE FULL CAPACITY OF THAT MEMBER AT THE POINT
- OF SPLICE. MEMBERS SHALL NOT BE SPLICED AT THE MAXIMUM STRESS POINTS. NO SPLICE IS TO BE MADE UNLESS SHOWN ON THE DRAWINGS OR REVIEWED AND APPROVED BY THE CONSULTANT.

10. PROVIDE WELDED STIFFENERS PLATES ON BOTH SIDES OF BEAMS AT POINTS OF CONCENTRATED

- LOADS INCLUDING BEAMS SUPPORTING COLUMNS OR RUNNING OVER THE TOP OF COLUMNS. 11. ANCHOR BOLTS TO ASTM A307, UNLESS NOTED OTHERWISE.
- 12.GROUT (NON-SHRINK) UNDER BEARING PLATES SHALL BE M-BED BY STERNSON LTD. OR
- 14.ALL FIELD WELDED CONNECTIONS TO BE INSPECTED BY FABRICATION CONTRACTOR.

<u>MATERIAL TESTING AND INSPECTIONS:</u>

15. PROVIDE WEEP HOLES IN ALL HSS STRUCTURAL.

AND INSPECTION COMPANY TO COMPLETE:

- 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENGAGING THE SERVICES OF A CERTIFIED TESTING
 - COMPACTION TESTING ON ALL INSTALLED GRANULAR MATERIAL. TESTING OF ALL PLACED CONCRETE/CEMENTITIOUS GROUT
 - IN ACCORDANCE WITH CAN/CSA-A23.2-14
 - ALL OTHER QUALITY CONTROL REQUIREMENTS AS OUTLINED IN OPS SPECIFICATIONS.

POSTED TENSIONED ANCHOR DOWEL:

• ROCK ANCHOR PULL TEST.

1. THE WORK TO BE DONE SHALL COMPRISE THE SUPPLY OF ALL LABOUR, PLANT AND MATERIAL AND THE PERFORMANCE OF ALL WORK NECESSARY FOR SUPPLYING, DRILLING, INSTALLATION, AND GROUTING, POST-TENSIONING TO THE DESIGN LOAD FOR THE ANCHOR DOWELS, AS SHOWN ON

ROCK DOWEL INSTALLATION:

CONTRACTOR'S EXPENSE.

INSERTION OF CEMENT GROUT PLUG.

TEXTURE, AND SHAPE CONDITIONS.

TOP/BOTTOM CLEARANCE AND/OR GROUT PLUG.

- I. ALL ROCK DOWEL LOCATIONS AND ORIENTATIONS SHALL BE COORDINATED WITH THE ENGINEER PRIOR TO DRILLING. BORE HOLES DRILLED WITHOUT PRIOR APPROVAL FROM THE ENGINEER MAY BE REJECTED AT THE DISCRETION OF THE ENGINEER. REJECTED BORE HOLES SHALL BE FILLED WITH GROUT AND NEW HOLES SHALL BE DRILLED AT THE CONTRACTOR'S EXPENSE.
- 3. UNLESS OTHERWISE SPECIFIED AND APPROVED IN WRITING BY THE ENGINEER. ALL BORE HOLES SHALL BE DRILLED USING A LOW IMPACT ROTARY DRILL TO PREVENT DAMAGE TO THE EXISTING DAM CONCRETE STRUCTURE. AIR POWERED DRILLS ARE NOT ALLOWED TO PREVENT DAMAGE TO

CORRECTIVE. CORRECTIVE ACTION AS SPECIFIED BY THE ENGINEER WILL BE COMPLETED AT THE

- 4. ALL BORE HOLES SHALL BE CLEANED OUT USING COMPRESSED AIR PRIOR TO INSERTING THE REINFORCING DOWEL. THE ENGINEER RESERVES THE RIGHT TO WITNESS THE CLEANING PROCESS AND INSERTION OF DOWELS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN ADVANCE OF CLEANING AND INSERTION OF DOWELS SO THAT DEPTH AND CLEANLINESS OF THE DRILLED HOLES CAN BE CONFIRMED. DOWELS GROUTED WITHOUT INSPECTION SHALL BE REJECTED AND
- REPLACED AT NO ADDITIONAL COST TO THE CLIENT. 5. ALL ROCK DOWELS SHALL BE GROUTED IN THE PRESENCE OF THE ENGINEER. GROUTING PERFORMED WITHOUT THE ENGINEER PRESENT SHALL BE REJECTED AND THE ROCK DOWELS REPLACED AT THE CONTRACTOR'S EXPENSE.
- 6. ALL ROCK DOWELS SHALL BE CENTERED IN THE DRILLED HOLE USING CENTRALIZERS CONTRACTOR SHALL ADJUST CENTRALIZER SIZE ACCORDING TO DRILLED HOLE DIAMETER IF NECESSARY. ANY ROCK DOWEL NOT CENTERED SHALL BE REJECTED AND REPLACED AT THE CONTRACTOR'S EXPENSE. ANCHORS OR DOWELS SHALL NOT DEVIATE MORE THAN 8% OF TH LED DIAMETER FROM THE CENTER OF THE HOLE. ALL DOWELS SHALL BE APPROVED PRIOR TO
- 7. THE CONTRACTOR SHALL KEEP THE AREA CLEAN DURING GROUTING. ANY SPILLED GROUT SHALL BE CLEANED IMMEDIATELY AT NO ADDITIONAL COST TO THE CLIENT. 8. SURFACE GROUT SHALL BE SCULPTED AND STAINED TO MATCH EXISTING SURROUNDING COLOR,

9. ROCK DOWEL MATERIAL, DIAMETER AND EMBEDMENT SHALL BE AS SPECIFIED ON THE DESIGN

DRILL DEPTH TO MEET THE REQUIRED MINIMUM EMBEDMENT LENGTHS AND ACCOUNT FOR

DRAWING. EMBEDMENT LENGTH SHALL BE THE ACTUAL LENGTH OF BAR SET BENEATH THE

EXISTING CONCRETE SURFACE AND ENCASED INTO GROUT. THE CONTRACTOR SHALL ADJUST THE

R-10 RIP RAP -

(400mm THICK MIN.)

CONCRETE NOTES:

1. CONCRETE IS SPECIFIED USING ALTERNATIVE NUMBER OF CSA A23.1 TABLE 2 AS

TABLE 1: EXTERIOR SLAB

- 7 (1 (7 (1 t) E 1 E 1 C 1 C 1 C 1 C 1 C 1 C 1 C 1 C 1	EXTERNOT OF THE
EXPOSED CLASS (TABLE 1)	C-2
AIR CONTENT CATEGORY (TABLE 4)	1
MAX. W/C RATION (TABLE 2)	0.5
CURING TYPE (TABLE 2)	2

- 7. ALL BOLTED CONNECTIONS TO BE BEARING TYPE. THE BOLTS SHALL BE BROUGHT TO TO A SNUG 2. AT LEAST TWO (2) WEEKS PRIOR TO THE PLACEMENT OF CONCRETE THE CONTRACTOR
 - A VALID 'CERTIFICATE OF READY MIXED CONCRETE PRODUCTION FACILITIES' OR VALID 'CERTIFICATE OF MOBILE MIX CONCRETE PRODUCTION FACILITIES' AS ISSUEI BY THE 'READY MIXED CONCRETE ASSOCIATION OF ONTARIO' TO THE PLANT BEIN
 - A COMPLETED 'CONCRETE MIXED DESIGN SUBMISSION FORM'

 - AIR VOID SYSTEM OF HARDENED CONCRETE.
 - STATISTICAL STRENGTH TEST ANALYSIS TO CONFIRM THE STRENGTH LEVEL FOR EACH CLASS OF CONCRETE INCLUDING THE EXPECTED 7/28 DAY STRENGTH RATIO
- 13.NO DRILLING OR CUTTING AFTER FABRICATION OF STRUCTURAL STEEL UNLESS APPROVED BY THE 3. FABRICATION AND PLACEMENT OF REINFORCING STEEL TO BE IN ACCORDANCE WITH CSA
 - 4. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO CSA G30.18 M92,
 - 5. CONCRETE SHALL NOT BE POURED UNTIL REBAR HAS BEEN INSPECTED BY THE ENGINEE
 - 7. CONCRETE COVER FOR REINFORCING STEEL AS PER TABLE 17 OF CSA A23.1

 - ALL CONCRETE CAST IN FORMS BUT EXPOSED TO EARTH OR WEATHER 50mm (• BARS 20M AND SMALLER IN WALLS AND SLABS 32mm (1 1/4")
 - CONCRETE NOT EXPOSED TO EARTH AND WEATHER 25mm (1") OR 1.5 NOMINAL BAR DIAMETER

8. ALL REINFORCING BARS SHALL BE ACCURATELY PLACED AND ADEQUATELY SUPPORTED

BY PRECAST CONCRETE, ADDITIONAL BARS, STIRRUPS, TIES OR APPROVED CHAIRS

- 9. CONTINUOUS AND TEMPERATURE REINFORCING BARS SPLICES TO BE LAPPED 40 BAR DIAMETERS OR A MINIMUM OF 300mm (12") AT SPLICES AND CORNERS, UNLESS OTHERWISE SHOWN ON SECTION OR TYPICAL DETAILS. LAP CONTINUOUS TOP BARS AT CENTRE BETWEEN SUPPORTS AND CONTINUOUS BOTTOM BARS AT SUPPORTS. AS
- 10. CONSTRUCTION JOINTS: SLAB CONSTRUCTION JOINTS MAY BE MADE ONLY AT APPROVE LOCATIONS SHOWN OR AS DIRECTED BY THE ENGINEER.
- 1. ALL CONCRETE ANCHORS TO BE HILTI HAS ROD HOT DIPPED GALVANIZED (DIAMETER A 2. THE LOCATES FOR DRILLING SHALL BE CLEAR OF BURIED UTILITIES. IT IS THE CONTRACTOR'S
- 3. ALL EPOXY ADHESIVE SHALL BE HILTI HIT-HY 200/HIT ICE. THE EXISTING CONCRETE STRUCTURE. ANY DAMAGE SHALL BE REPORTED TO THE ENGINEER FOR

4. INSTALL AS PER MANUFACTURER'S SPECIFICATIONS.

1. ROCK DOWELS TO BE:

AGAINST DISPLACEMENT.

- 1. CEMENTITIOUS GROUT FOR ROCK DOWELS TO BE SIKAGROUT 212. 2. CEMENTITIOUS GROUT UNDER BASE PLATES TO BE SIKAGROUT 212.
- **ROCK DOWELS:**
- 15M SPILLWAY WALL, DAM RAISING, DAN EXTENSION AND RETAINING WALL #2 ALL DOWELS TO CONFORM TO CSA G30.18 M92, Fy=400 MPa (GRADE 400)

75mm TOPSOIL

AND HYDROSEED

ENGINEER'S SEAL:



5/06/2025 ISSUED FOR CONSTRUCTION ISSUED FOR TENDER ISSUED FOR APPROVALS ISSUED FOR FINAL REVIEW 8/05/2024 3 ISSUED FOR CLIENT REVIEW ISSUED FOR CLIENT REVIEW 5/04/2024 2 8/03/2024 ISSUED FOR CLIENT REVIEW 0/09/2022 0 ISSUED FOR CLIENT REVIEW REVISION

DATE REV.

Township of Muskoka Lakes 1 Bailey Street, P.O. Box 129

Port Carling, Ontario

LITTLE BURGESS

REHAB

DESIGNED | CHECKED | APPROVED DRAWN **JUNE 25, 2025 SCALE**

RIP RAP DRAINAGE SWALE X-SECTION

SCALE 1:50

SHALL SUBMIT THE FOLLOWING TO THE ENGINEER FOR REVIEW:

- A QUALITY PLAN THAT DESIGNATES A SPECIFIED SLUMP OR SOME OTHER MEASURE OF WORKMANSHIP
- TEST RESULTS TO SHOW COMPLIANCE WITH CSA A23.1.
- A23.1 AND THE REINFORCING STEEL INSTITUTE OF CANADA'S 'REINFORCING STEEL MANUAL OF STANDARD PRACTICE'.
- Fy=400 MPa (GRADE 400)
- 6. WHERE REBARS JOIN AT CORNERS PROVIDE CORNER BARS FOR LAPS.
- ALL CONCRETE CAST AGAINST EARTH 75mm (3")
- REQUIRED, TERMINATE CONTINUOUS BARS AT NON CONTINUOUS ENDS WITH STANDARD
- CONCRETE ANCHORS:
- 2. ALL CONCRETE ANCHORS TO BE COMPLETE WITH HOT DIPPED GALVANIZED FLAT WASHERS, NUTS, LOCK NUTS, ETC. AS SPECIFIED ON THE DRAWINGS.

- 25M POWER HOUSE STABILIZATION 20M - RETAINING WALL

2. ALL DRILLED HOLES FOR ROCK DOWELS SHALL BE 25mm GREATER THAN THE DOWEL

GENERATING STATION

GENERAL NOTES

PROJECT No. REVISION DRAWING

FOR STABILITY. STOCKPILED MATERIALS SHALL BE PROTECTED WITHOUT CONTAMINATION AND EROSION.

THE DAM SITE APPROVED BY THE CLIENT OR ITS REPRESENTATIVE. STOCKPILES SHALL BE SLOPED SUFFICIENTLY